

GEOPHYSICAL INVESTIGATION REPORT

Concept Engineering Consultants

Clinton Commons Project

Clinton, New Jersey

December 23, 2022 (REV. 4)



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1 Executive Summary

ANS Geo was retained by Concept Engineering Consultants to complete a broad-scale geophysical investigation to evaluate potential karst conditions at the proposed Clinton Commons project site located in the Town of Clinton, New Jersey. Our geophysical survey was completed as a supplemental investigation behind a previous investigation completed by Engineering and Land Planning Associates in June 2009 and April 2020, and ahead of an ANS Geo's 2022 Geotechnical investigation consisting of percussion probes and test borings. Through review of Engineering & Land Planning Associates 2020 "Karstic Geology Investigation Report", USGS NAPP color infra-red (CIR) imagery was evaluated and eight (8) possible karstic locations were delineated based off of that imagery.

Based on our review of available information, we identified the project site is mapped by the United States Geological Survey (USGS) as being underlain by the Allentown Formation and Lower Beekmantown Group consisting primarily of Dolomite with some Shale and Orthoquartzite bedding. Dolomite bedrock, while not typically as prone as Limestone, can be generally susceptible to karst. To better evaluate the presence or absence of karst anomalies at the project site, ANS Geo completed a geophysical investigation program consisting of Electrical Resistivity Imaging (ERI), to characterize the type, depth, and extent of karst features at representative locations across the site. The geophysical program was not an exhaustive evaluation of the entire site, but intended to gain a general understanding of the subsurface conditions and the impact of karst on the design, siting, and construction of the project.

ANS Geo completed the ERI geophysical survey at the project site on February 28 and March 1, 2022. In total, nine (9) ERI survey transects were completed at locations depicted as potential karst zones as well as along a northwest-southeast running fault line and within the projects planned SWM Recharge Basin.

The surveys conducted generally showed steep trends in depths between upper soil horizons, weathered bedrock, and competent bedrock. Survey interpretations identified interbedded upper clay and soil-like residuum and possible "epi-karst" consisting of a gravel-clay-sand mixture. These soil-like residuum zones were sporadic and were observed through analysis of ERI results, to different degrees, within all the profiles surveyed. Weathered top of bedrock was generally observed between ground surface and approximately 5 to 10 feet below grade. As expected, our surveys indicate that the quality of the bedrock generally improves with increased depth. Top of bedrock was moderately to highly pinnacled or abruptly changing in depth, with particular locations exhibiting possible deep soil or soil-residuum horizons, most likely caused by deep weathering of the bedrock over time and possible karst zones.

Based on our preliminary evaluation of the geophysics results, it appeared that karst may be of low-risk to design and construction within the broader project boundaries. Karst features, such as pinnacled top of bedrock, and areas of possible soil infilling were observed within the majority of the ERI profiles. Therefore, these existing conditions shall be considered for the proposed foundations of structures and design and location of proposed stormwater basin.

Apparent resistivity values above approximately 10,000 ohm-meters can generally be categorized as possible "air-filled" karst anomalies. These values can also be associated with "massive" bedrock, or extremely fractured bedrock. Zones depicting bedrock, then decreasing below resistivities of 100 ohm-meters may represent clay or soil-infilled anomalies.

To further investigate and confirm ERI survey results, ANS Geo completed a Geotechnical Investigation Program between May 2022 and September 2022 consisting of 12 test borings and ten percussion probes. The detailed summary of the findings is included in Section 3.2 and 3.3.



2 Introduction

ANS Geo was retained by Concept Engineering Consultants to assist with Phase II investigation program as requested by Engineer for Town of Clinton. The Town Engineer's request included the following:

- Borings shall include 10-foot rock cores as described in the Ordinance to properly assess the condition of the underlying site bedrock.
- All identified sinkhole locations shall be investigated, since all eight (8) of potential areas are either
 within the footprints of the proposed structures or within proximity of the proposed stormwater
 basin.
- The northeast corner of the proposed Food Market where two (2) USGS mapped fracture traces and a fault intersect will require further assessment.
- The plan proposes several deep cuts (18 ft. +/-) for sewer utility installation near the proposed northern site entrance. The soil and bedrock conditions along those alignments need to be investigated to assess the potential impact of the installation.
- A major area of concern is near the single SWM Recharge Basin that is proposed for this project. The Phase I study identified possible sinkholes on three (3) sides of the proposed stormwater basin and as such, the bedrock condition underlying the proposed stormwater basin requires a thorough investigation to assess any potential impacts. This area is of high concern given its proximity to residential structures and lack of any explorations into the bedrock. Consideration must be given to the fact that more than one SWM Basin may be required (ref: G-3) to meet the Highlands requirements.

To be cost efficient, ANS Geo proposed completing non-invasive geophysical investigation to evaluate karst conditions at the proposed Clinton Commons project site in the Town of Clinton, New Jersey prior to performing test borings and/or test pits at specific locations. As part of our initial review process prior to mobilizing on the project site, ANS Geo reviewed geotechnical reports from previous investigations on the project, which identified certain areas prone to the potential for karst features such as sinkholes and faults within the project boundary.

ANS Geo placed Electrical Resistivity Imaging (ERI) locations at localized locations within the project site to obtain a specified set of data which was used to characterize the type, depth, and extent of karst features at select representative locations across the site. It should be noted the purpose of the geophysical program was not intended to be an exhaustive evaluation of the entirety of the site, as that intent would require extensive and comprehensive canvassing and investigation across the entirety of the project site. However, the investigation was intended to gain a general understanding of the subsurface conditions near locations identified in previous investigations by others and requested by the Town Engineer to gauge the impact which karst geohazards may or may not contribute to the design, siting, and construction of the proposed project. **Figure 1** below depicts a project vicinity map.



Figure 1: Project Vicinity Map

(Source: Google Earth Imagery 2021)

Field Investigation

3.1 Electrical Resistivity Imaging (ERI)

ANS Geo completed ERI survey at the project site on February 28th and March 1st, 2022. A Geophysics Investigation Location Plan, which shows the location of all geophysical survey transects (lines) as they correspond to the proposed site development layout is provided as Appendix A. ERI survey methods used for this field investigation were a combination of Dipole-Dipole and Strong Gradient. The methods were completed using an array of electrodes positioned in a linear fashion along the proposed survey locations. In total, nine (9) ERI survey transects were completed within the project area. Appendix B includes Figures 1 through 6, which represent each of the nine surveyed locations with an associated profile. ANS Geo's ERI surveys were cross-referenced against previous test boring data completed by Engineering and Land Planning Associates. Table 1 summarizes the geophysical survey method and ID number, reporting Figure number, distance, and orientation of each line, as well as their spacing distances used for ERI testing.

Method-ID	Figure No.	Table 1 – Geophysical S	Survey	Electrode	Electrode Spacing
			Distance (ft)	Configuration/Qty	(ft)
ERI-1	1	North to south	270	28	10
ERI-2	1	Northwest to southeast	270	28	10
ERI-3	2	Northwest to southeast	270	28	10
ERI-4	3	Northwest to southeast	270	28	10
ERI-5	4	Northwest to southeast	270	28	10
ERI-6	4	Southwest to northeast	275	56	5
ERI-7	5	Southwest to northeast	270	28	10
ERI-8	5	West to east	560	56	10
ERI-9	6	North to south	270	28	10



3.2 Percussion Probes

The percussion probes were completed by Hayduk Enterprises of Factoryville, Pennsylvania between May 4th, and May 10th, 2022. Percussion probes were advanced using ECM-590 Self-Contained Hydraulic Crawler Drill, which uses a drilling hammer with compressed air and a down-the-hole hammer with drilling bit that is advanced by this hammering and rotation action. All percussion probes were advanced a depth of 49 feet BGS and estimated top of rock is based on drilling timing is provided in **Table 2** below.

Estimated Completed Percussion Top of Rock **Depth** Probe ID (feet) (feet) PP-01 49 7 PP-02 7 49 PP-03 7 49 PP-04 4 49 PP-05 24 49 PP-06 7 49 PP-07 12 49 PP-08 18 49 PP-09 6 49 PP-10 5 49

Table 2 - Percussion Probe

Sudden drops of drilling rod, which is a typical indication of karst features such as air-void or soil-filled void were not encountered in all completed percussions probes. The ERI survey results from ERI-5, ERI-6, ERI-8, and ERI-9 indicated overburden soil to be thicker than other ERI survey locations. This was confirmed by percussion probes PP-05, PP-07 and PP-08, where the overburden soil was encountered as deep as 24 feet BGS, 12 feet BGS, and 18 feet BGS, respectively. All percussion probes were backfilled as per NJDEP well abandonment requirements. Investigation Location Plan and percussion probe logs are provided in **Appendix C and Appendix D**, respectively.

3.3 Test Borings

ANS Geo retained Boring Brothers, Inc. of Egg Harbor, New Jersey to advance the test borings. The first mobilization of test borings was completed between May 11th and May 13th, 2022, and the second mobilization of test boring were completed between September 12th and September 20th, 2022 using a CME-55LC track-mounted drill rig with a 3-7/8-inch diameter tri-cone roller bit mud-rotary techniques to the proposed borehole termination depth or top of rock. Once estimated bedrock was encountered, minimum 10 feet of rock coring was performed in accordance with Town of Clinton's Chapter 88 Land Use Article VII Zoning Regulations 88-64.2 Carbonate Area District requirements. Soil samples were collected using the Standard Penetration Test (SPT) Method in accordance with American Society for Testing Materials (ASTM) Standard D1586 – Standard Test Method for SPT and Split-Barrel Sampling of soils. Rock coring was completed using ASTM D2113-08 – Standard Practice for Rock Core Drilling. All boreholes were backfilled as per NJDEP well abandonment requirements. It should be noted that NJDEP requested that test borings shall not be performed within 1,000 feet of existing bald eagle's nest once ANS Geo's test boring crew mobilized on site in May 2022. Therefore, only six test borings were completed in the previous report submitted by ANS Geo. As of this report, remaining six test Borings were completed in September 2022. This report comprises of all the Borings which is summarized in Table 3 below.



Table 3 - Test Borings

Borehole ID	Approx. Existing Elevation (feet)	Approx. Proposed Elevation (feet)	Approx. Elevation Difference (feet)	Proposed Boring Depth (feet)	Encountered Top of Rock (feet)	Total Depth of Rock Coring (feet)	Borehole Termination Depth (feet)
B-01	235	241	-6	40	17	10	27
B-02	234	241	-7	40	Borehole Ren	noved from So	ope of Work
B-03	253	256	-3	40	20	10	30
B-04	222	224	-2	40	23	10	33
B-05	246	246	0	40	4	10	14
B-06	269	264	5	40	10	30	40
B-07	245	244	1	40	5	10	15
B-08	264	263	1	40	10	10	20
B-09	249	242	7	40	3	10	13
B-10	258	256	2	40	10	10	20
B-11	222	224	-2	40	Borehole Ren	noved from So	ope of Work
B-12	260	261	-1	40	10	10	20
B-13	235	225	10	40	10	10	20
B-14	245	251	6	40	Borehole Ren	noved from So	cope of Work
B-15	253	252	1	40	4	10	14

3.3.1 Encountered Subsurface Conditions in Test Borings

Total 12 of 15 proposed test borings were completed in this report. Three borings were removed from our scope of work due to sufficient test borings and percussion probes. As completed boring locations are included in the Investigation Location Plan in **Appendix C**. The overburden material encountered consisted of sand and clay underlain by gravel stratum before encountering bedrock. Average N-values ranged from 6 to 15 blows per foot (bpf) within the sand and clay stratums, and greater than 50 bpf within the gravel stratum. Groundwater was not encountered within the overburden soil.

Top of bedrock within the completed twelve test borings ranged from 4 to 23 feet BGS. Recovered rock cores were classified as Limestone moderately weathered, weak to medium strong rock, and very close to close discontinuities spacing. Rock core recovery ranged from 13% to 100% and Rock Quality Designation (RQD) ranged from 0% to 97%. Fractured rock zones were generally encountered within the low RQD zones.

Based on ERI-4 survey results, potential karst anomaly may exist between 25 to 37 feet BGS on the southern end of the ERI survey. Therefore, test boring B-06 was advanced to minimum 40 feet BGS at the location to determine if karst features such as soil infilled or air-void will be encountered. In test boring B-06, top of bedrock was encountered at 10 feet BGS and 30 feet of bedrock was cored. Rock core recoveries ranged from 98% to 100% and RQDs ranged from 45% to 97%. Fractured rock was encountered between 31.3 to 34.4 feet BGS, but loss of drilling water, drill rod drops, or residual soil zones were not encountered within test boring B-06, which are typical indications of karst anomalies. In addition, ANS Geo attempted use a borehole camera to confirm any anomalies, but water in the open borehole prevented recording any clear images of cored borehole. For additional details, refer to **Appendix E** for test boring logs and rock core photos.



4 Geophysical Investigation Method

4.1 Electrical Resistivity Imaging (ERI)

ERI is a geophysical survey method that measures electrical resistivity in soil and rock based off the principles of Ohm's Law. Data obtained through an ERI investigation acquires a series of voltage and current measurements from surface electrode arrays. The electrode arrays consist of a series of dipoles that communicate with other dipoles. The arrays can be spaced close or very far apart depending on necessary survey resolution. Resistivity is dependent on the material property and geometry and thus is measured in Ohm-meters.

4.1.1 Theory

Electrical resistance is based upon Ohm's Law:

$$R = \frac{V}{I}$$
 [ohms]

Where, resistance, **R**, is equal to the ratio of potential, **V** (volts) to current flow, **I** (amperes).

Resistivity is the measure of the resistance along a linear distance of a material with a known cross-sectional area. Consequently, resistivity is measured in Ohm-meters. This Report presents the geophysical results as geo-electrical profiles of modeled resistance plotted as two-dimensional profiles of distance and depth, in units of feet.

Electrical currents propagate as a function of three material properties: (1) ohmic conductivity, (2) electrolytic conductivity, and (3) dielectric conductivity. Ohmic conductivity is a property exhibited by metals. Electrolytic conductivity is a function of the concentration of total dissolved solids and chlorides in the groundwater that exists in the pore spaces of a material. Dielectric conductivity is a function of the permittivity of the matrix of the material. Therefore, the matrix of most soil and bedrock is highly resistive. Of these three properties, electrolytic conductivity is the dominant material characteristic that influences the apparent resistivity values collected by this method. In general, resistivity values decrease in water-bearing rocks and soil with increasing:

- a. Fractional volume of the rock occupied by groundwater;
- b. Total dissolved solid and chloride content of the groundwater;
- c. Permeability of the pore spaces; and,
- d. Temperature.

Materials with minimal primary pore space (i.e., limestone, dolomite) or those which lack groundwater in the pore spaces will exhibit high resistivity values (Mooney, 1980). Factors contributing to low resistivity:

- Degree of water bearing void space within soil and rock (only if water exists);
- Chloride content of water bearing within soil and rock pore space;
- Available pore space within material (i.e., low pore space will decrease resistivity);
- Temperature.

Highly porous, moist, or saturated soil will exhibit very low resistivity values. Additionally, high resistivity values will result from generally inverse conditions (i.e., highly-porous and dry conditions). This is, of course, a range, and most earthen materials falls within the range of low to medium resistivity depending on their properties. For these reasons, cavities, voids, highly fractured bedrock and groundwater can often have definable values observed through the methods of ERI.



In homogeneous ground, the apparent resistivity is the true ground resistivity; however, in heterogeneous ground, the apparent resistivity represents a weighted average of all formations through which the current passes.

4.1.2 Methods

Different acquisition algorithms can be implemented during an investigation. For this investigation, the Dipole-Dipole / Strong-Gradient array combination methods, which have proven to be an effective configuration for imaging voids in shallow bedrock settings, were implemented. The measurements were collected to create a two-dimensional image. The image is developed using an inversion algorithm. The inversion algorithm uses the collected apparent resistivity data to create a model space of resistivity values that would replicate the collected data.

While homogeneous ground conditions represent the true apparent ground resistivity, non-unique values represent a weighted average of the multiple formation variations (Reynolds, 1997). Apparent resistivity values are computed with a forward modelling subroutine, and a smoothness-constrained least-squares optimization routine, creating a pseudosection using finite-difference or finite-element approaches. The pseudosection model is compared to the actual measurements for consistency. A measure of the inversion progress and difference is given by the root-mean-squared error.

4.1.3 Data Collection and Data Processing

Six total ERI profiles were acquired using an AGI SuperSting R8 Resistivity meter. Seven (7) of the ERI surveys were completed with a 28-electrode setup at 10-feet spacings. One (1) ERI profile was acquired in conjunction with a 56-electrode setup at 5-foot spacings and another at 10-foot spacings. Measurements were obtained through a combined Dipole-Dipole and Strong Gradient paired array setup. Locational data were recorded using a Trimble Geo7X global positioning system. The approximate depth of penetration of the survey is contingent on a few factors, most of which relate to the overall survey line length. Each test reached an approximate penetration depth of 60 feet below existing grade. Two-dimensional profiles have been provided within **Appendix B**.

5 Background Geology

Prior to site mobilization, ANS Geo reviewed geologic mapping made publicly available by the United States Geologic Survey (USGS), which indicates the site is underlain by Allentown Dolomite and the Lower Beekmantown Group. These groups both generally consist of light gray to medium-gray dolomite with minor orthoquartzite and shale beds. These rock types are known for their susceptibility of karst formation which is addressed in Section 5.3.

5.1 Surficial Geology

ANS Geo also conducted a desktop study of the surficial geology in the project area using the National Resource Conservation Service (NRCS) Web Soil Survey. The Web Soil Survey only evaluates the upper five feet of soils, as it is generally used for agricultural purposes. However, reviewing this information can indicate what soil properties can be expected on site. The NRCS mapping indicates that the project area consists primarily as material of the Duffield silt loam unit, which is comprised primarily of silts and clays and shallow unweathered bedrock.

5.2 Bedrock Geology

A desktop review of the local bedrock geology was conducted using publicly available mapping and literature published by the New Jersey Geological and Water Survey and the USGS. Based on this mapping, the predominant bedrock formation within the project boundary is the Allentown Formation consisting primarily of Dolomite. The Lower Beekmantown Group exists within the southwestern portion of



the site and also consists of Dolomite. Due to the degree of folding and fracturing of the bedrock, bedrock may generally present a high degree of dipping. Additionally, a thrust-fault was mapped within the northwest corner of the project site heading southeast. After reviewing the New Jersey Geological Society's latest (2015) *Bedrock Geologic Map of the High Bridge Quadrangle*, it appears that the previously mapped "Thrust Fault" as depicted within E&LP's Report has been updated and moved southwest of the project site. The updated mapping does place a thrust fault within the proposed developments. However, it now runs northwest to southeast along the southwest boundaries of the project site, as depicted within **Figure 2** below. In addition, Concealed Fault, Anticline, and Syncline are mapped within the project site.

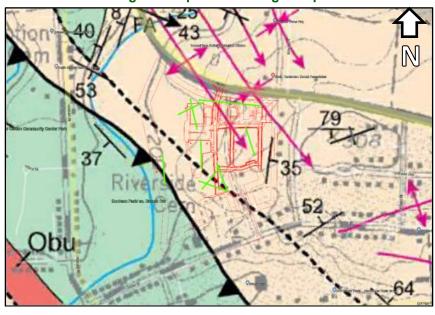


Figure 2: Updated Geologic Map

Based on our knowledge and experience, concealed faults can be small and are difficult to identify compared to thrust faults. The "dipping" direction or dip angle is unknown compared to identified thrust faults unless bedrock is exposed above ground surface.

5.3 Karst Geology

Ground subsidence, commonly referred to as "sinkholes", is the local downward movement of surface material with little or no horizontal movement. Subsidence is a potential geologic hazard in areas where karst terrain occurs, or where underground mining has taken place. In karst terrain, limestone and dolomite bedrock (carbonate rock formations) are eroded by water and create karst features such as subsurface channels, caves, and sinkholes. Within the Allentown Formation, karst can be prevalent. Due to the project site having multiple mapped fracture sets, these are areas where a higher amount of dissolution may occur as they become preferential pathways for groundwater drainage.

5.4 Aerial Imagery Evaluation for Previous On-Site Subsidence Events

ANS Geo conducted a review of aerial images across the site to create a map of potential subsidence events that have occurred or are currently active within the project boundaries. Potential subsidence incidences can be identified by reviewing site topography, looking at shading on the ground surface of aerial images, surface water drainage pathways, and looking for pooling or standing water. No identifiable subsidence occurrence within the project boundaries could be confirmed visually via aerial imagery. Through review of Engineering & Land Planning Associates 2020 "Karstic Geology Investigation Report", USGS NAPP color infra-red (CIR) imagery was evaluated, and eight (8) possible karstic locations were delineated based off of that imagery. Those locations are depicted in **Figure 3**.





Figure 3 - Potential On-Site Subsidence Incidents

(Source: Google Earth 2021 Imagery)

6 Geophysical Analysis

Limestone and Dolomite that matures within karstic or dissolution prone conditions undergoes a variable maturation process. When younger, the features represent small caves, short caves, and uniform rockhead. As the karst matures, so does its complexity. Cover-subsidence and cover-collapse sinkholes, irregular or pinnacled rockhead, buried sinkholes, all become more prevalent. The surveys conducted showed variability in the presence, depth, and characteristics of karst features across the site; however, they were also consistent on multiple fronts.

6.1 Electrical Resistivity Imaging Analysis

The depth to interpreted bedrock ranged from approximately at existing grade to 10 feet below existing grade with the results of the ERI surveys and previous completed soil borings correlating well (showing similar depths). ANS Geo's planned geotechnical investigation will provide more data from test borings and/or test pits to correlate and confirm ERI results.

Top of bedrock was observed to generally fluctuate along the ERI profiles. This is indicative of "pinnacled" top of bedrock and usually occurs over extended dissolution and weathering of the bedrock surface. As expected, our surveys indicate that the quality of the bedrock generally gets better with increased depth. There were indications of past dissolution, collapse and soil-infilling within a six of the ERI surveys completed. As these zones are soil-infilled and have already disintegrated, it is in our opinion that they will not provide a large risk to the project's development.

ERI methods provide indications of overall stratigraphy type and change, possible anomalies such as voids or caverns, and water bearing zones. Apparent resistivity values obtained through the ERI surveys



portrayed variable subsurface conditions with apparent resistivities ranging from less than 1 ohm-meters to over 13,000 ohm-meters. The subsurface profiles generally exhibited a moist lower resistivity clay and silt layer within the upper approximately 5 to 10 feet below grade which then varied in material type between decomposed dolomite and zones of variable resistivities within the upper approximately 5 to 30 feet below grade. A zone of very high (>10,000 ohm-m) resistivity within the upper 12 to 327 feet below existing grade existed within ERI-4. Competent bedrock was generally observed with increasing depth.

6.2 Geology Analysis

A top layer of clay with frequent areas of gravel inclusion was generally observed within the five to 30 feet below existing grade. Where subsidence has occurred, these soils can be very loose, indicating raveling of soils (into previously-formed voids) with one moderate sized possible open void. Particular trends were observed within the ERI data showing that portions of profiles may have experienced some degree of "raveling". These zones will typically exhibit lower bearing strengths as the soils have experienced loosening due to possible subsidence in the past. This upper soil transitioned into a weathered dolomite that has predominantly weathered to clay, silt and gravel with sections of intact rock. This zone of weathered bedrock extended to variable depths and had transitions to pinnacled top of rock with abrupt change.

No indications of surface depressions were visible at the time of our ERI surveys.

7 Risk Evaluation and Conclusions

ANS Geo understands that the project site is intended to support commercial development, which will consist of residential buildings and commercial buildings such as retail stores, food market store, gas station and convenience store. In addition, new development supporting systems such as stormwater recharge basin, water lines, gas line, and stormwater and sewer lines are proposed to be constructed. To aid in site planning and development, it is important to identify the relative potential for risk across certain portions of the site to help minimize the potential for siting critical project components and structures (i.e. building foundations) within these areas with higher geologic risk of settlement and movement.

Through our investigation, it does not appear that significant representations of sinkholes or air-filled karst appear within the ERI survey data. However, karst features such as pinnacled top of bedrock, and areas of potential sinking and infilling were observed. Percussion probes and test borings were completed at select locations to confirm the presence or lack thereof karst features depicted within the geophysics results. The follow-up investigation consisting of percussion probes and test borings did not indicate that any of these features exist. Typically, if a feature exists, while drilling, drilling water loss or a sudden drop of drilling rods or soft zones would be encountered. Drilling water loss, rod drops, and soft zones were not encountered in the completed percussion probes and test borings. Additionally, the previously mapped fault locations, depicted within ERI-5 and ERI-6 were looked at closer during the geotechnical subsurface investigation consisting of a percussion probe along the two geophysical surveyed lines. The probe did not indicate any rod drops or clear indications of subsurface variation. Due to this location's proximity to a nearby bald eagle's nest and as per NJDEP's request, no test borings within confirmatory rock core were completed at time of this report.

Through our preliminary evaluation of geophysics survey results, it is in our opinion that there are karstic features onsite; however, they appear to be relic and soil-infilled features. These karstic conditions should be considered while designing foundations for the proposed developments and planning for the stormwater basin. **Table 4** provides a summary of the inferred depth to bedrock, as well as subsurface profile, evaluated as part of our geophysical investigation.



Table 4 – Geophysics Survey Notes

Geophysics ID	Topographic & Geologic Setting	Inferred Approximate Depth to Top of Bedrock [feet]	Notes
ERI-1	Mild sloping	~ 0' ~ 20'	Clay/Silt overburden layer with possible inclusions of gravel and sand generally 0 to 20 feet thick. Bedrock abruptly changes with depth "pinnacled". Bedrock quality is variable across the survey's length and depth.
ERI-2	Moderate sloping	~ 3' – 10'	Shallow bedrock that abruptly changes in depth.
ERI-3	Moderate sloping	~ 5' – 10'	Pinnacled top of bedrock. Generally shallow competent rock. A possible dissolution and weathered rock zone exists at approximately 190 to 270 feet horizontal distance along the ERI line.
ERI-4 Moderate sloping		~ 0' – 10'	Discontinuous overburden soils with shallow bedrock. The bedrock is highly pinnacled.
ERI-5	Mild sloping	~ 3' – 25'	Bedrock dips steeply from the northwest to southeast. Fine-grained material present as overburden soil. A possible discontinuous zone of gravel or floating bedrock exists between 3 to 25 feet. The bedrock appears to dip northwest to southeast at an approximate depth of 20 to 40 feet along the ERI line. No indications of a fault were represented within the data or follow-up percussion probe completed.
ERI-6	Mild sloping	~ 3' – 20'	Overburden soils appear to be "epi-karst" with remnants of fine- grained soils as well as granular soil and floating bedrock. Competent bedrock appears at an approximate depth of 20 to 30 feet below grade.
ERI-7 Moderate sloping		~ 0' – 10'	Bedrock quality fluctuates along the horizontal and vertical extents of the ERI survey. A highly weathered zone exists at an approximate horizontal distance of 110 to 190 feet.
ERI-8	Mild sloping	~ 0' – 30'	There may be a deep soil horizon up to 30 feet deep. The soils would most likely be a combination of fine-grained and coarse-grained material including bedrock remnants.
ERI-9	Steep sloping	~ 3' – 25'	Abruptly changing top of bedrock. Multiple dissolution and soil-infilled zones exist across the extents of the survey line.

8 Limitations

ANS Geo notes that the findings and recommendations presented with this Report are based on investigation program completed by ANS Geo between February and September 2022, and our engineering judgement. Geophysical investigations are a non-invasive method of interpreting physical properties of the shallow earth using electrical, electromagnetic, or mechanical energy. This document contains geophysical interpretations of responses to induced or real-world phenomena. As such, the measured phenomenon may be impacted by variables not readily identified in the field that can result in a false-positive and/or false negative interpretations. ANS Geo makes no representations or warranties as to the accuracy of the interpretations. The extent of reliability of the survey is based on the specific areas where surveys were conducted; areas outside surveyed alignments may have variations in the conditions noted. We also understand that the current investigation is considered preliminary, and that traditional geotechnical investigations including an appropriate number of borings, and associated laboratory testing of soil material have been or will be completed prior to detailed design and construction.



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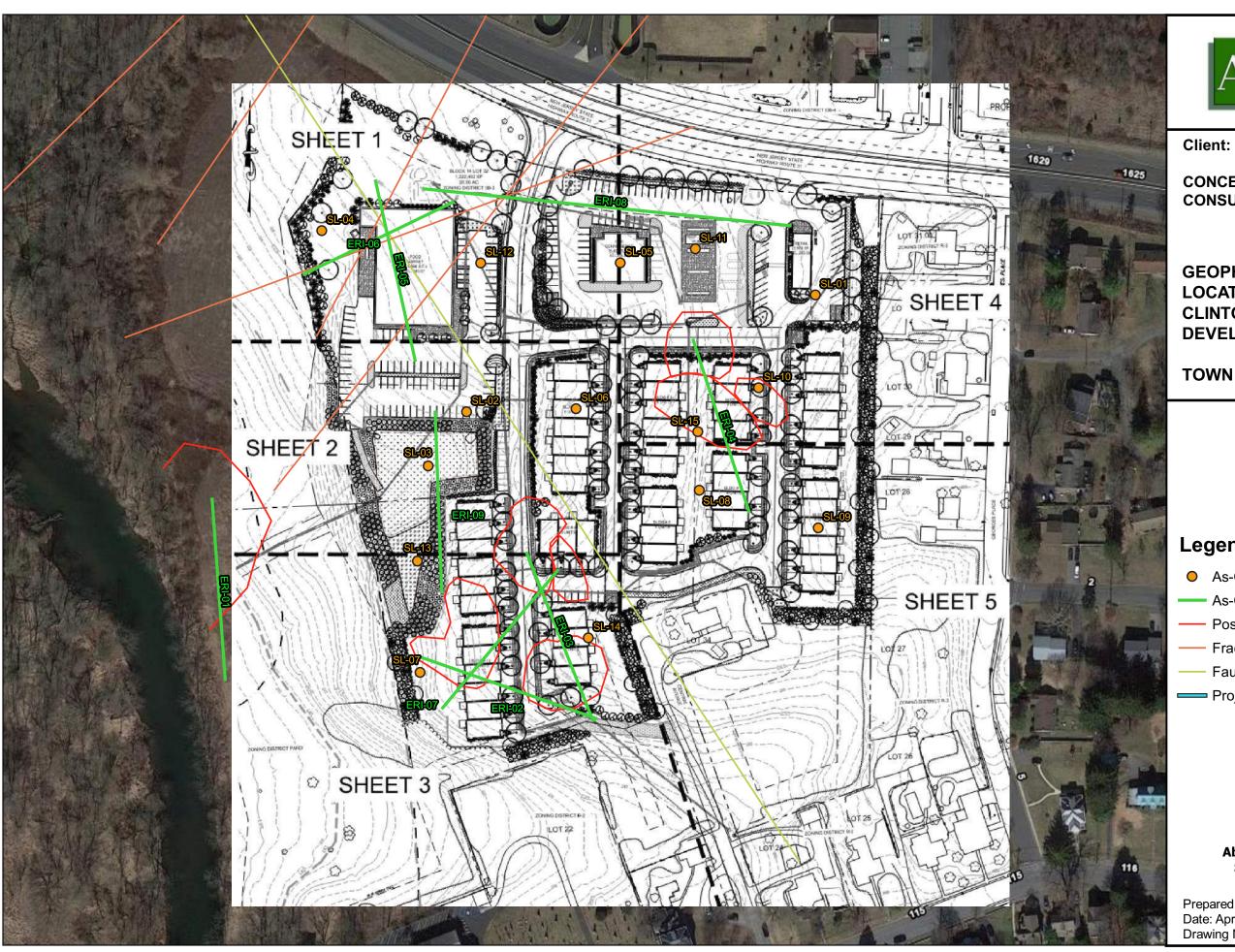
Mooney, H.M. (1980) Handbook of Engineering Geophysics: Volume 2: Electrical Resistivity, Bison Instruments, Inc.

Reynolds, J.M. (1997) An Introduction to Applied and Environmental Geophysics, John Wiley, Chichester, New York, 796 p.

Xia, J.; Miller, R. D.; Park, C. B.; Hunter J. A.; Harris, J. B.; and Ivanov, J., (2002), Comparing shearwave velocity profiles inverted from multi-channel surface wave with borehole measurements: *Soil Dynamics Earthquake Engineering*, Vol. 22, pp. 181-190.

APPENDIX A

Geophysical Investigation Survey Location Plans





CONCEPT ENGINEERING CONSULTANTS, PA

GEOPHYSICS INVESTIGATION LOCATION PLAN CLINTON COMMONS DEVELOPMENT

TOWN OF CLINTON, NEW JERSEY

Legend

- As-Completed E&LP Soil Borings
- As-Completed Geophysics Locations
- Possible Karst Locations
- Fractures as per E&LP's Report
- Fault as per E&LP's Report
- Project Boundary

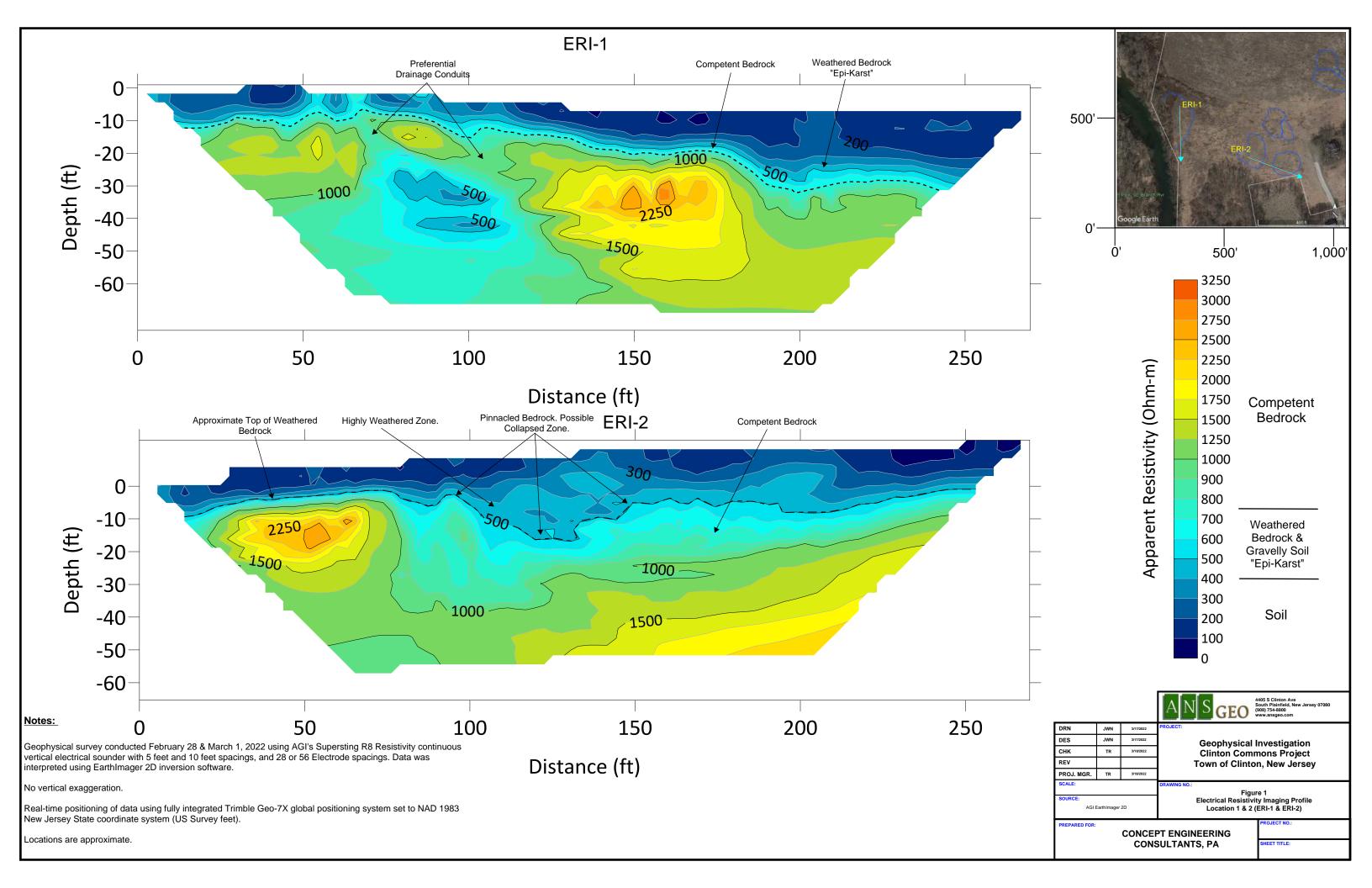
140 ft

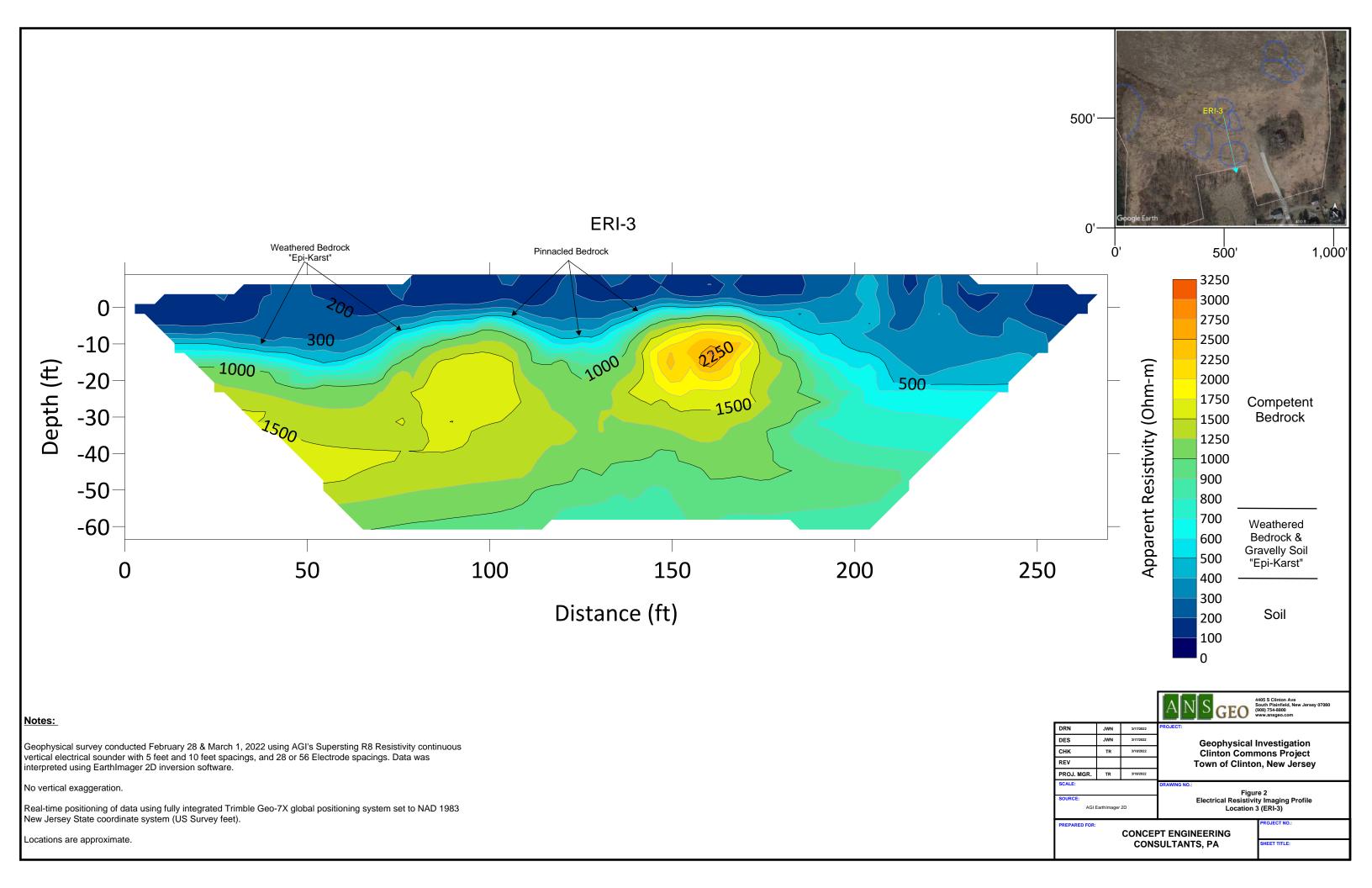
Absolute Scale: 1 inch = 150 feet Scale at 11" x 17" AS SHOWN

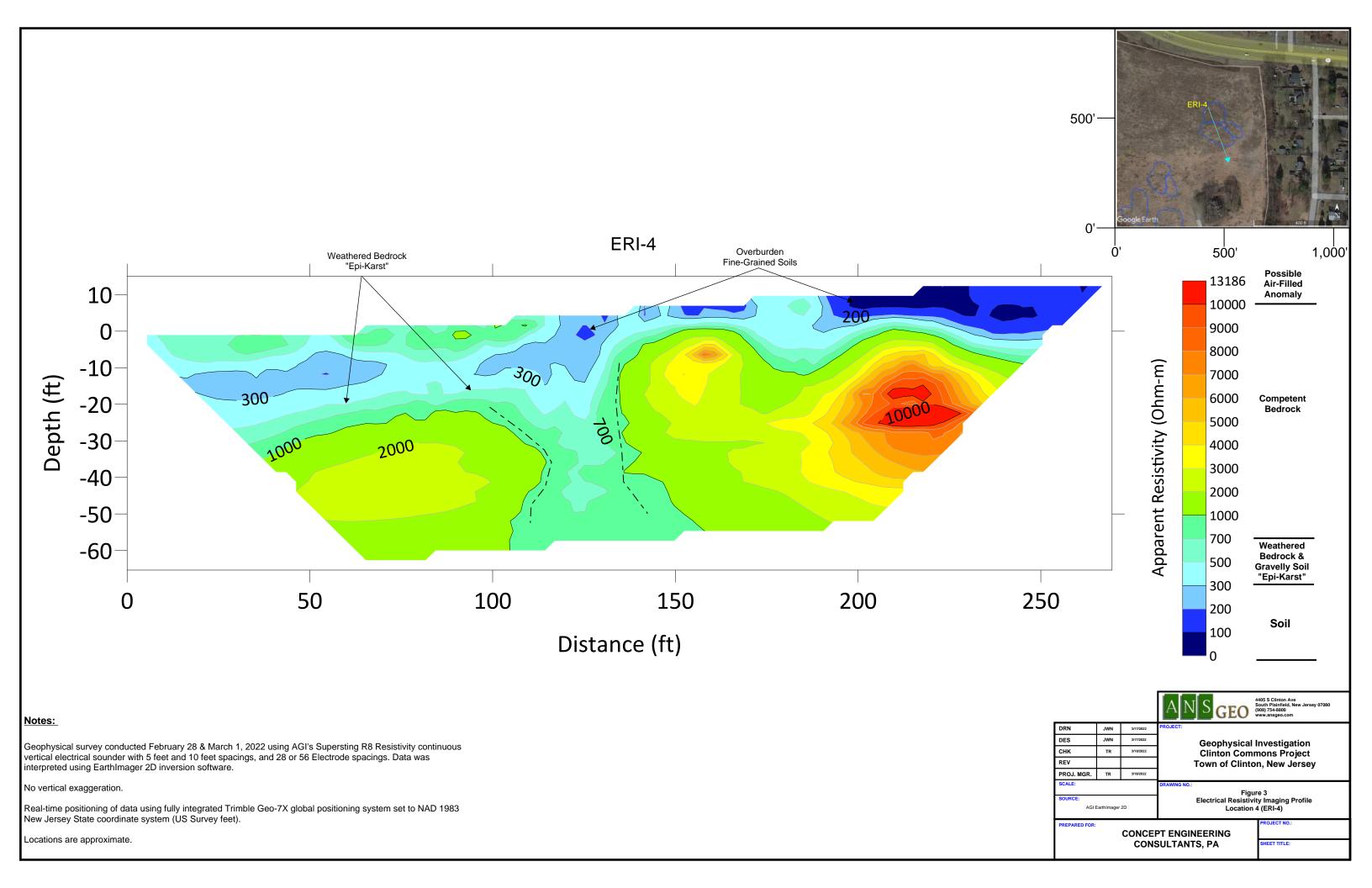
Prepared by: Jonathan Nelson Date: April 7, 2022 Drawing Number: PIP-1 Rev.1

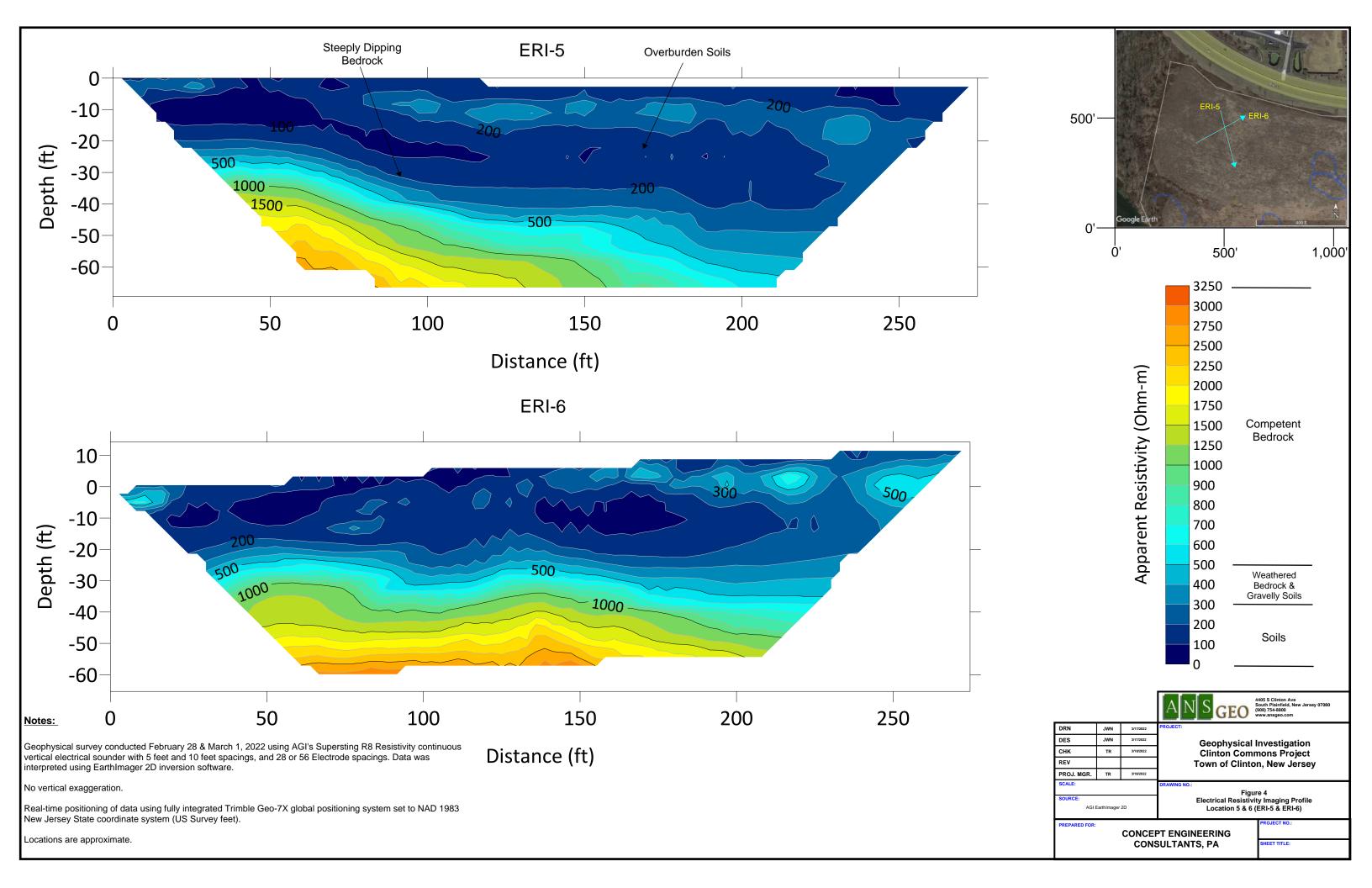


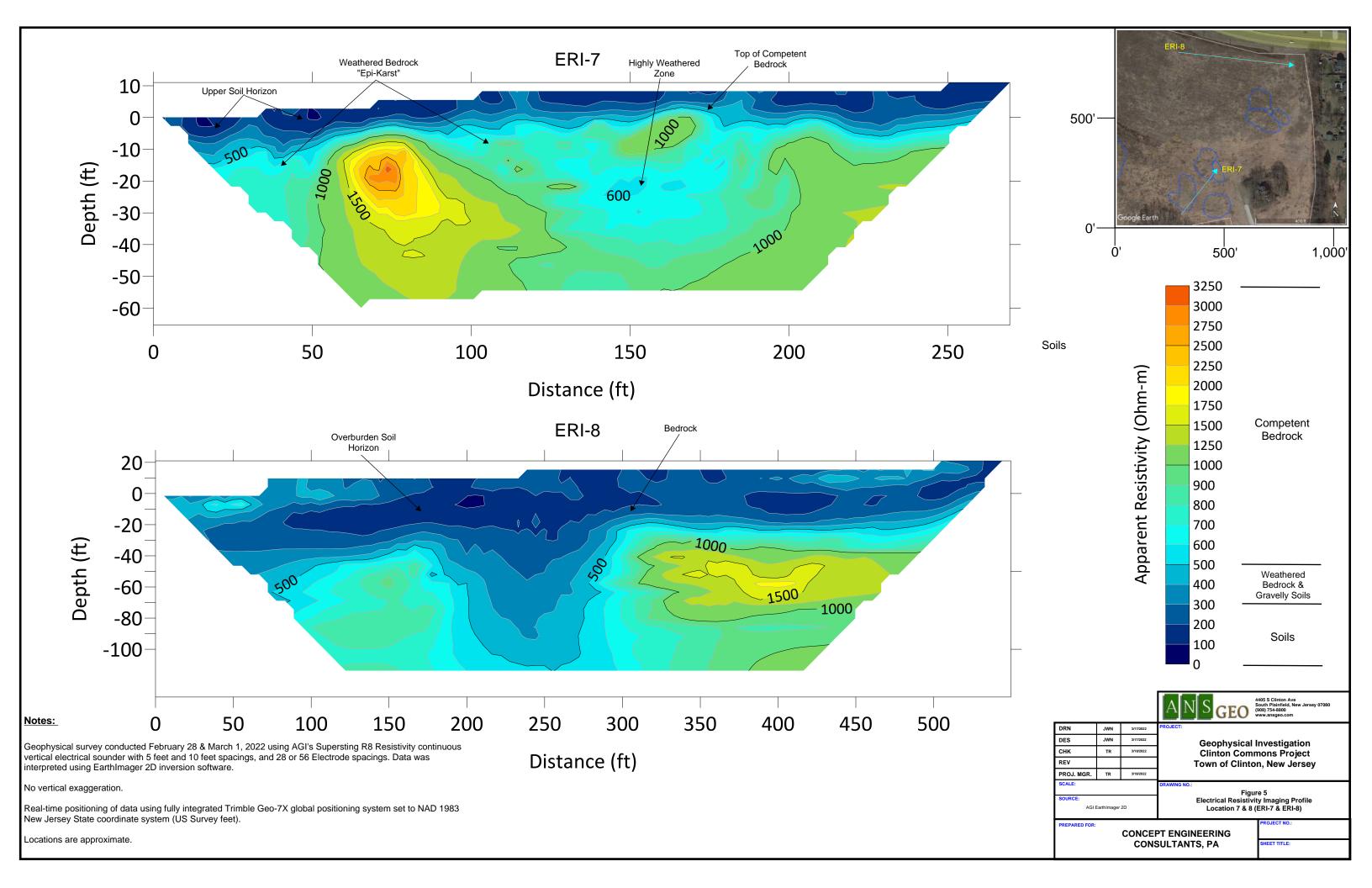
APPENDIX B Electrical Resistivity Imaging Profiles

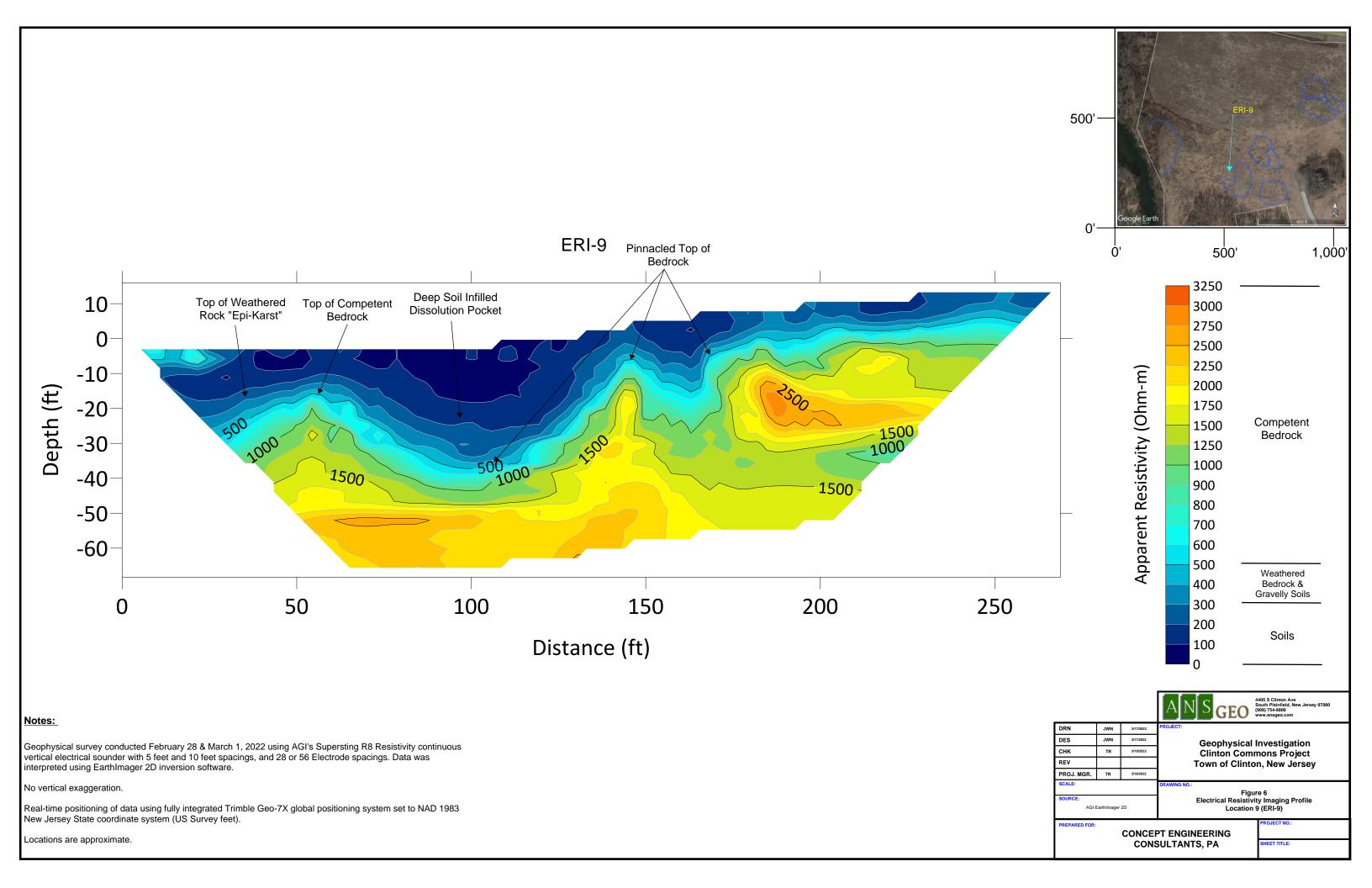








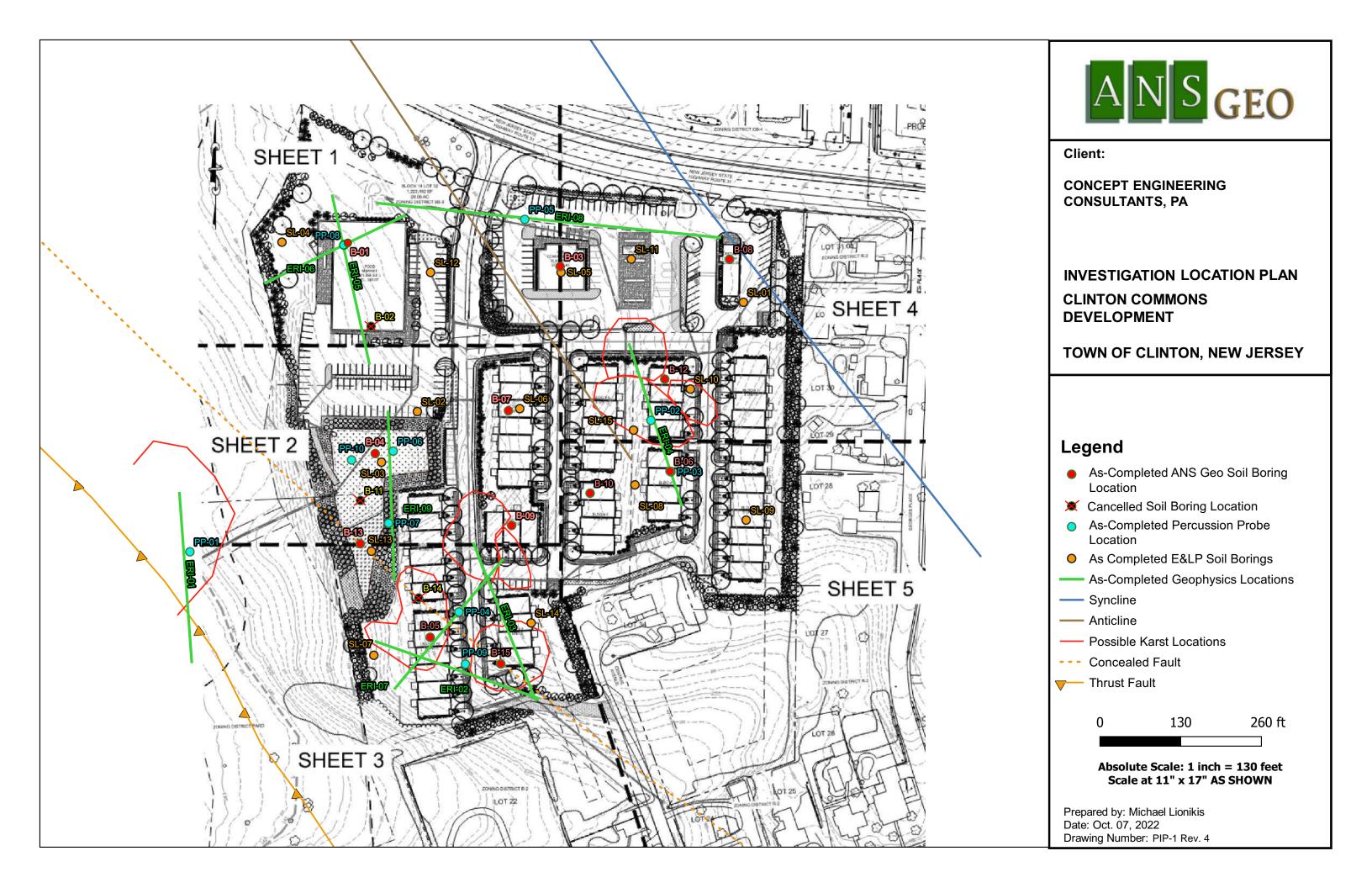






APPENDIX C

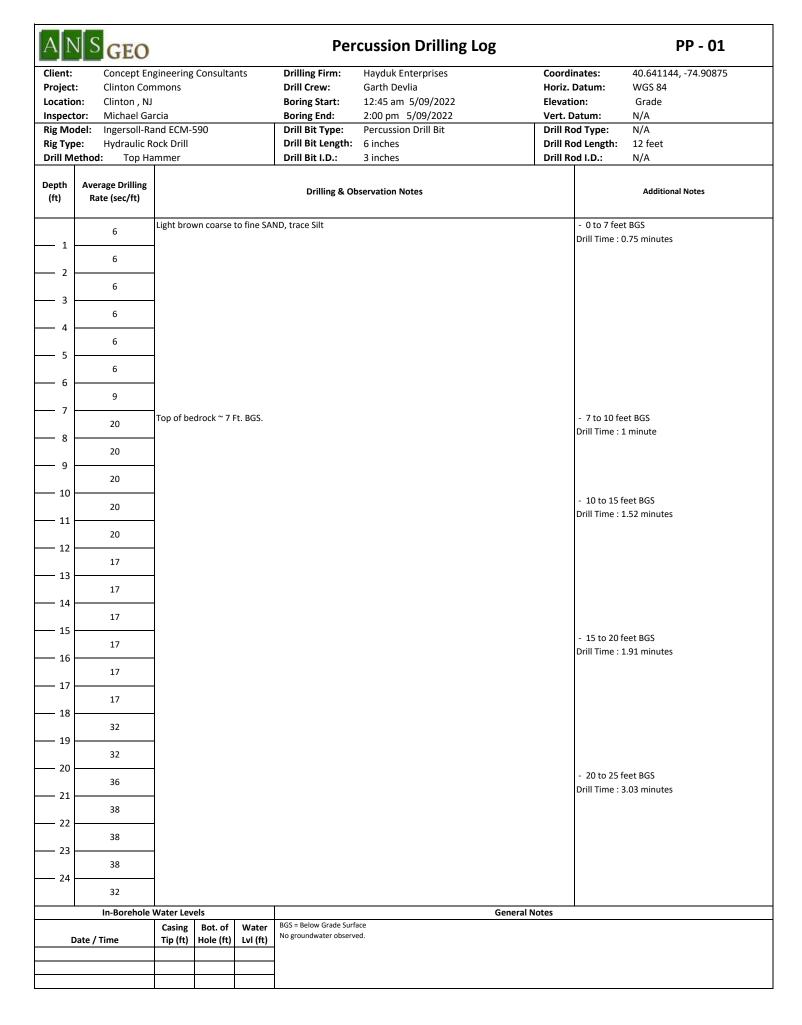
Investigation Location Plan





APPENDIX D

As-Completed Percussion Probe Logs





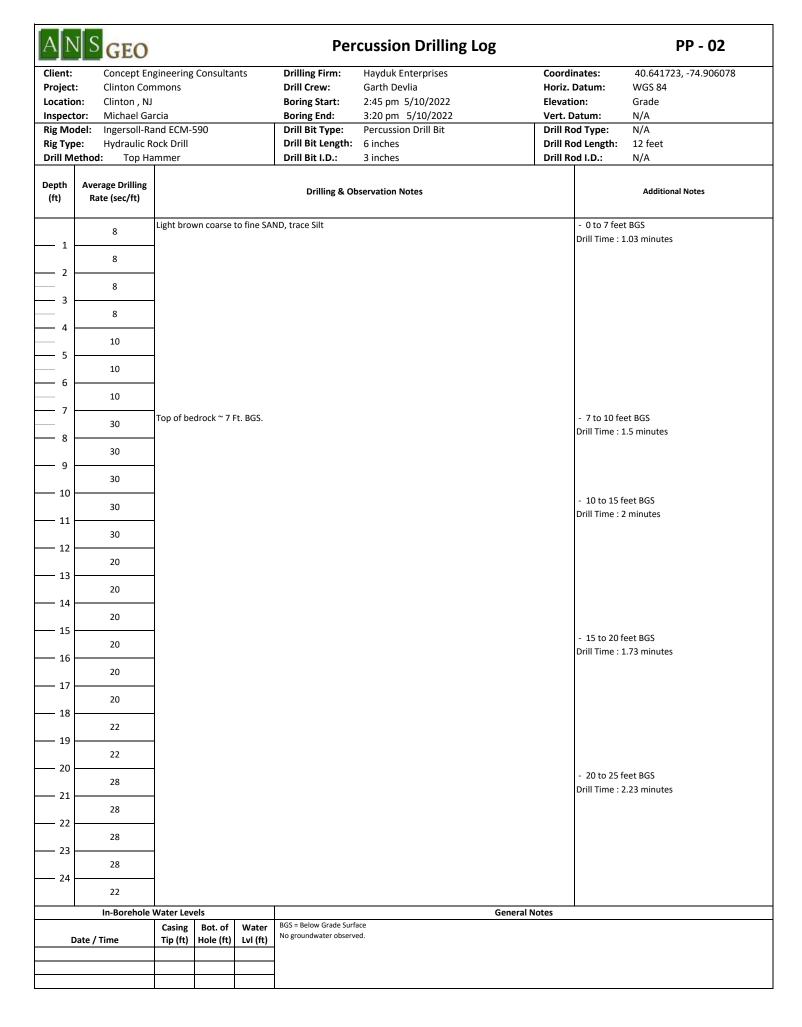
Percussion Drilling Log

PP - 01

continued)

Client: **Drilling Firm:** Hayduk Enterprises 40.641144, -74.90875 **Concept Engineering Consultants** Coordinates: Project: **Clinton Commons Drill Crew:** Garth Devlia WGS 84 Horiz. Datum: **Boring Start:** 12:45 am 5/09/2022 Location: Clinton , NJ Elevation: Grade Michael Garcia 2:00 pm 5/09/2022 Inspector: **Boring End:** Vert. Datum: N/A

Oepth (ft)	Average Drilling Rate (sec/ft)				Drilling & Observation Notes	Additional Notes
	32					- 25 to 30 feet BGS Drill Time : 2.67 minutes
26	32					Dilli Time . 2.07 minutes
- 27	32					
— 28 –	32					
– 29 –	32					
— 30 -	30					- 30 to 35 feet BGS Drill Time : 2.92 minutes
— 31 —	30					Dilli Tillie . 2.92 Hilliutes
- 32 -	35					
— 33 —	40					
- 34	40					
— 35 –	40					- 35 to 40 feet BGS Drill Time : 3.67 minutes
- 36 -	45					
— 37 —	45					
— 38 —	45					
— 39 —	45					
- 40	42					- 40 to 45 feet BGS Drill Time : 3 minutes
- 41 - 42 -	42					
— 42 -	36					
— 43 - — 44 -	30					
— 44 — — 45 —	30					
— 43 — 46 –	30					- 45 to 49 feet BGS Drill Time : 2 minutes
— 46 — 47	30					
- 47 - 48	30					
— 49 —	30					Total Drill Time in Rocks : 21.72 minutes
— 50					19 feet BGS. entonite holeplug	
	In-Borehole	Nater Lev				ral Notes
Da	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Surface No groundwater observed.	





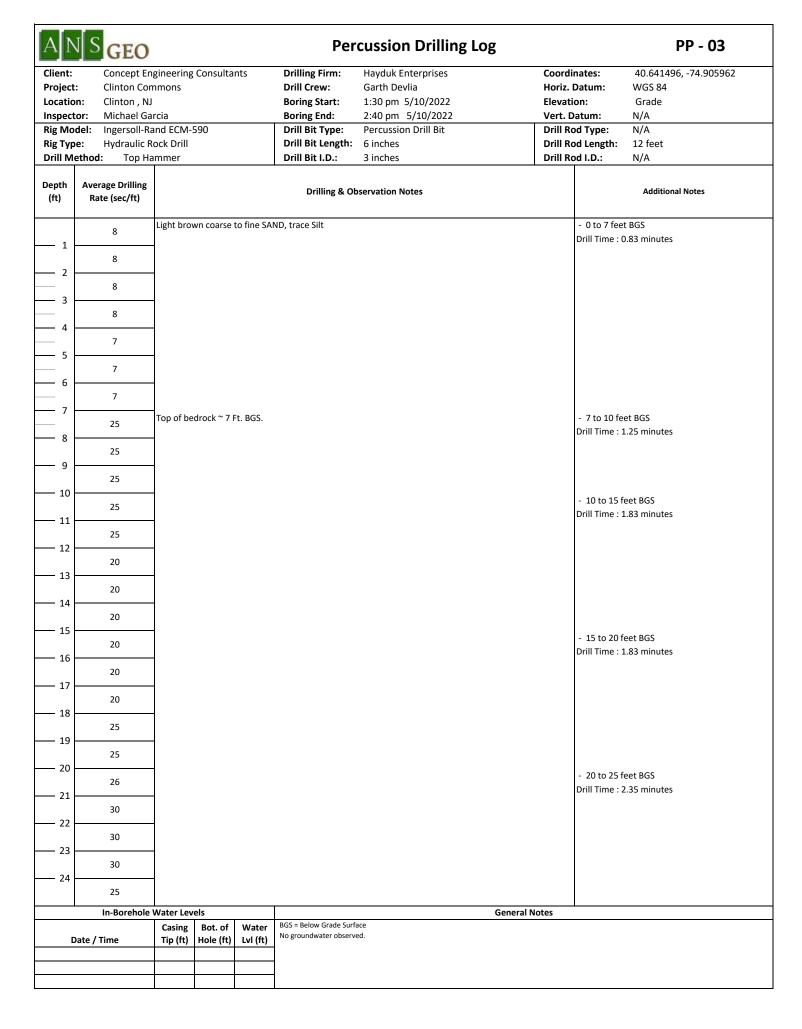
Percussion Drilling Log

PP - 02

ntinued)

Drilling Firm: Hayduk Enterprises 40.641723, -74.906078 Client: **Concept Engineering Consultants** Coordinates: Project: **Clinton Commons Drill Crew:** Garth Devlia WGS 84 Horiz. Datum: 2:45 pm 5/10/2022 Location: Clinton , NJ **Boring Start:** Elevation: Grade Michael Garcia **Boring End:** 3:20 pm 5/10/2022 Inspector: Vert. Datum: N/A

Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & Observation Notes	Additional Notes
	22					- 25 to 30 feet BGS Drill Time : 1.83 minutes
26	22					Jan Time : 1.05 minutes
27	22					
28	22					
29	22					
 30	20					- 30 to 35 feet BGS
 31	20					Drill Time: 1.83 minutes
 32	20					
 33	25					
 34	25					
 35	25					- 35 to 40 feet BGS
 36	30					Drill Time : 2.42 minutes
 37	30					
 38						
 39	30					
	30					- 40 to 45 feet BGS
 41	25					Drill Time : 1.97 minutes
 42	25					
 43	20					
	24					
— 45	24					- 45 to 49 feet BGS
	24					Drill Time: 1.6 minutes
	24					
<u> </u>	24					
	24	- 1 65				Total Drill Time in Rocks : 18.14 minutes
50					19 feet BGS. Jentonite holeplug	
	In-Borehole			ı	General Notes	
D	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Surface No groundwater observed.	





Percussion Drilling Log

PP - 03

Client: **Concept Engineering Consultants** Project:

Clinton Commons

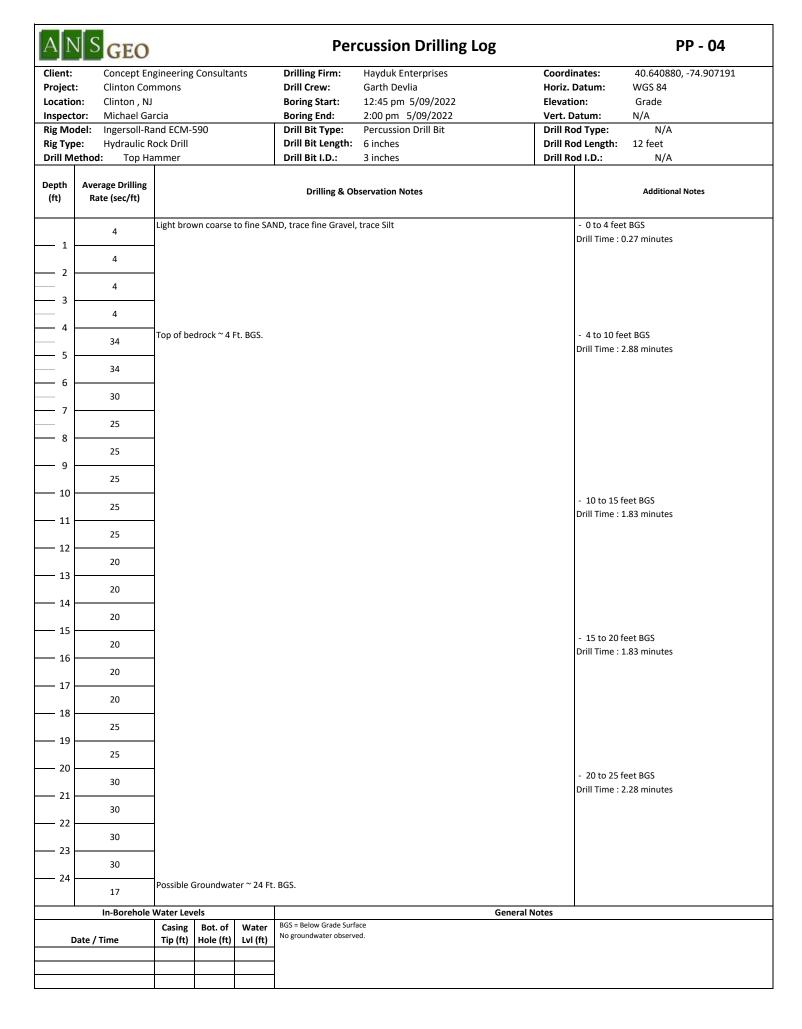
Drilling Firm: Hayduk Enterprises **Drill Crew:**

Garth Devlia 1:30 pm 5/10/2022 Coordinates: Horiz. Datum: 40.641496, -74.905962

WGS 84

Location: Clinton , NJ **Boring Start:** Elevation: Grade Michael Garcia **Boring End:** 2:40 pm 5/10/2022 Inspector: Vert. Datum: N/A

Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & Observation Notes	Additional Notes
	25					- 25 to 30 feet BGS Drill Time : 2.08 minutes
26	25					Dilli filile . 2.06 fililitates
27	25					
28	25					
 29						
30	25					- 30 to 35 feet BGS
 31	28					Drill Time : 2.68 minutes
32	28					
 33	35					
 34	35					
35	35					
	35					- 35 to 40 feet BGS Drill Time : 2.85 minutes
 36	34					
 37	34					
 38	34					
 39	34					
	40					- 40 to 45 feet BGS
 4 1	40					Drill Time : 3.17 minutes
 42	40					
 43						
	35					
 45	35					- 45 to 49 feet BGS
 46	35					Drill Time : 2.33 minutes
	35					
48	35					
— 49	35					Total Drill Time in Rocks : 21.20 minutes
					19 feet BGS. entonite holeplug	
<u> </u>	In-Borehole				General Notes	
n	ate / Time	Casing	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Surface No groundwater observed.	
	,			(14)		





Project:

Percussion Drilling Log

PP - 04

Client: Concept Engineering Consultants

Clinton Commons

Drilling Firm: Hayduk Enterprises
Drill Crew: Garth Devlia

Garth Devlia 12:45 pm 5/09/2022 Coordinates:

(continued) 40.640880, -74.907191

Location: Clinton , NJ **Inspector:** Michael Garcia

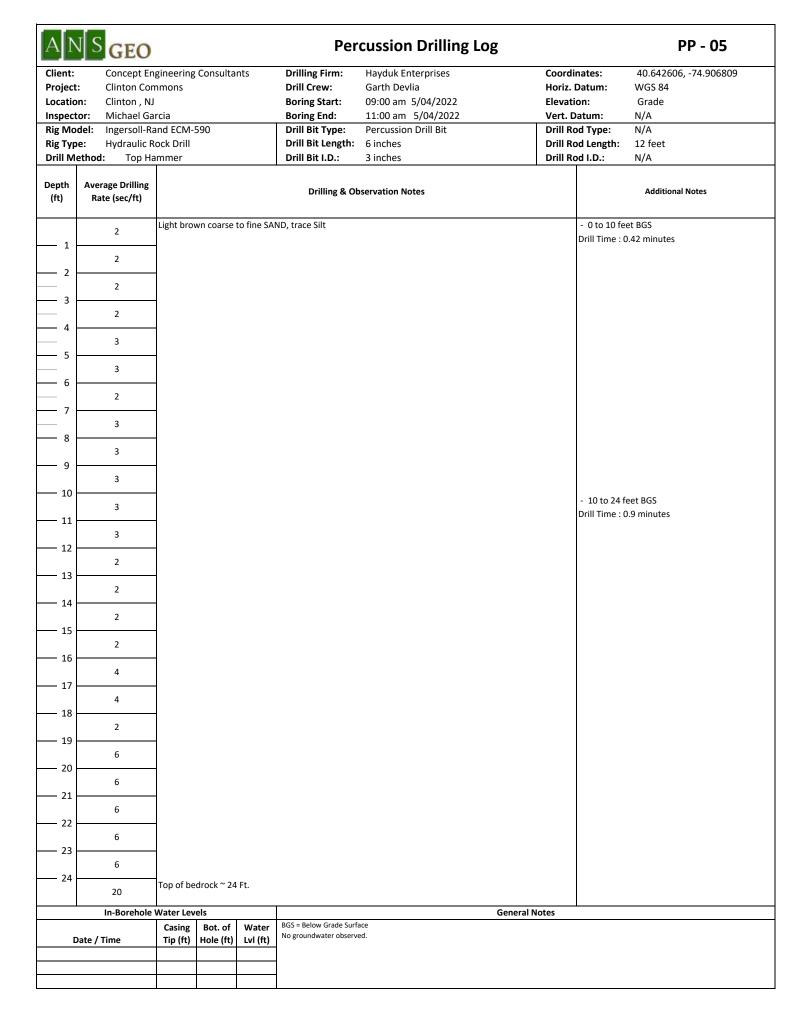
Boring Start: 12:45 pm 5/09/2022 **Boring End:** 2:00 pm 5/09/2022 Horiz. Datum: Elevation: Vert. Datum:

N/A

WGS 84

Grade

Inspect	or: Michael Ga	lCld			Boring End:	2:00 pm 5/09/2022	Vert. L	Jatum:	N/A
Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & C	Observation Notes			Additional Notes
26	17							- 25 to 30 Drill Time	feet BGS : 1.42 minutes
26	17								
27	17								
28 29	17								
30	17								
31	22							- 30 to 35 Drill Time	feet BGS : 1.98 minutes
32	22								
33	15								
—— 34 ·	30								
 35	30								
 36	30							- 35 to 40 Drill Time	1 feet BGS : 2.17 minutes
 37	25								
 38	25								
39	25								
	25							- 40 to 45	feet RGS
	22								: 1.67 minutes
	22								'
43	22								
 44	17								
 45	17							- 45 to 49	feet BGS
 46	17							Drill Time	: 1.13 minutes
	17								
48	17								
					9 feet BGS.			Total Drill T	ime in Rocks : 17.46 minutes
50				ings and b	entonite holeplug		Conoval Natas		
	In-Borehole	Casing	Bot. of	Water	BGS = Below Grade Su		General Notes		
D	ate / Time	Tip (ft)	Hole (ft)		No groundwater obser				





Location:

Inspector:

Percussion Drilling Log

PP - 05

Client: **Concept Engineering Consultants** Project:

Clinton Commons Clinton , NJ

Michael Garcia

Drilling Firm: Hayduk Enterprises **Drill Crew:** Garth Devlia

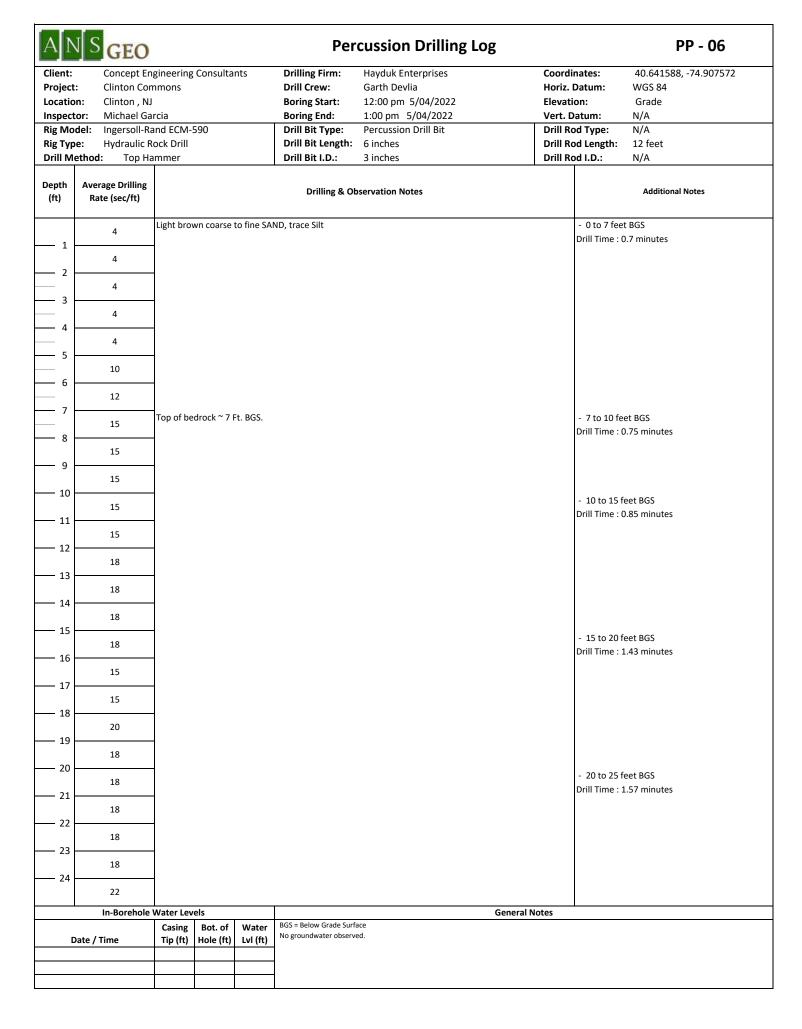
09:00 am 5/04/2022

Coordinates: Horiz. Datum:

40.642606, -74.906809 WGS 84

Boring Start: Elevation: Grade **Boring End:** 11:00 am 5/04/2022 Vert. Datum: N/A

Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & Observation Notes	Additional Notes
	20					- 24 to 30 feet BGS Drill Time : 2.0 minutes
26	20					Dill Time . 2.0 minutes
— 27	20					
— 28	20					
— 29						
— 30	20					- 30 to 35 feet BGS
— 31	15					Drill Time : 1.18 minutes
— 32	14					
— 33 -	14					
- 34	14					
— 35 -	14					
— 36 -	14					- 35 to 40 feet BGS Drill Time : 1.63 minutes
	21					
— 37 -	21					
— 38 -	21					
— 39 -	21					
- 40	26					- 40 to 45 feet BGS
— 41 -	26					Drill Time : 2.17 minutes
- 42	30					
- 43	24					
— 44						
– 45	24					- 45 to 49 feet BGS
- 46	24					Drill Time : 1.6 minutes
– 47	24					
– 48	24					
— 49	24					Total Drill Time in Rocks : 9.9 minutes
— 50					9 feet BGS. entonite holeplug	
	In-Borehole	Water Lev	rels			General Notes
D	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Surface No groundwater observed.	





Percussion Drilling Log

PP - 06

Client: **Concept Engineering Consultants** Project:

Clinton Commons

Drilling Firm: Hayduk Enterprises **Drill Crew:** Garth Devlia

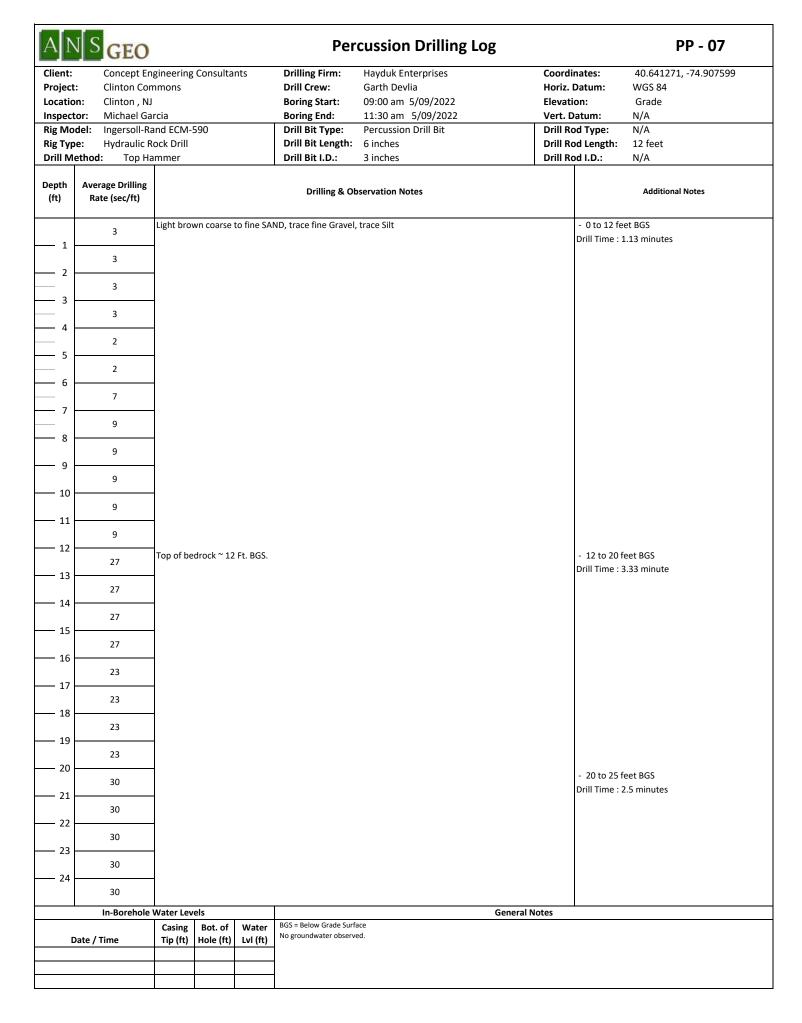
12:00 pm 5/04/2022

Coordinates: Horiz. Datum: 40.641588, -74.907572

WGS 84

Boring Start: Location: Clinton , NJ Elevation: Grade Michael Garcia 1:00 pm 5/04/2022 Inspector: **Boring End:** Vert. Datum: N/A

Depth	Average Drilling	Cla			Drilling & Observation Notes	Additional Notes
(ft)	Rate (sec/ft)				-	- 25 to 30 feet BGS
26	22					Drill Time : 1.83 minutes
	22					
 27	22					
28	22					
29						
30	22					- 30 to 35 feet BGS
31	22					Drill Time : 1.77 minutes
32	21					
	21					
 33	21					
 34	21					
 35	21					- 35 to 40 feet BGS
36						Drill Time: 2.08 minutes
 37	26					
38	26					
39	26					
	26					
	23					- 40 to 45 feet BGS Drill Time : 1.78 minutes
	23					Dim time . 1.70 timutes
 42	18					
 43						
 44	18					
 45	25					45 to 40 fort DCC
	25					- 45 to 49 feet BGS Drill Time : 1.67 minutes
	25					
	25					
 48	25					Total Drill Time in Rocks : 14.43 minutes
 49					9 feet BGS.	Total Drill Time III ROCKS : 14.43 minutes
50				ings and b	entonite holeplug	
	In-Borehole \			Water	General Note BGS = Below Grade Surface	s
D	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)		No groundwater observed.	





Location:

Inspector:

Percussion Drilling Log

PP - 07

Concept Engineering Consultants Project:

Michael Garcia

Clinton Commons Clinton , NJ

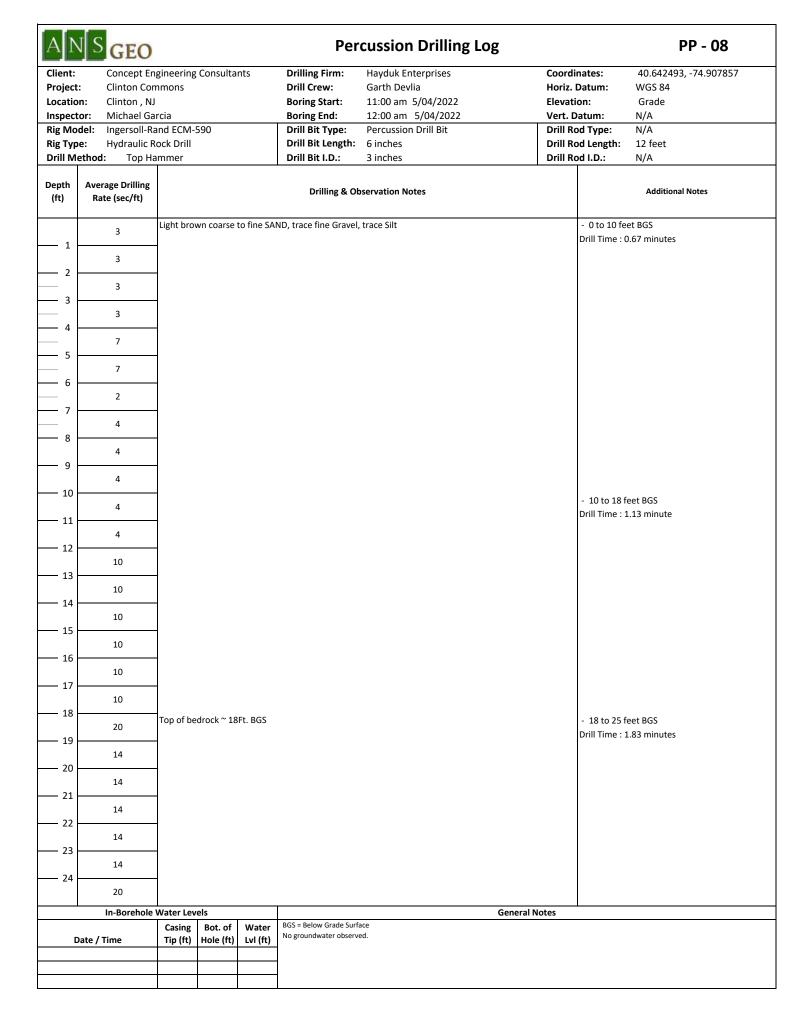
Drilling Firm: Hayduk Enterprises **Drill Crew:**

Garth Devlia 09:00 am 5/09/2022 Coordinates: Horiz. Datum:

40.641271, -74.907599 WGS 84

Boring Start: Elevation: Grade **Boring End:** 11:30 am 5/09/2022 Vert. Datum: N/A

Depth Average Drilling					Drilling & Observation Notes	vert. D	Additional Notes
(ft)	Rate (sec/ft)				Dinning & Observation Notes		
26	30						- 25 to 30 feet BGS Drill Time : 2.5 minutes
27	30						
28	30						
29	30						
30	30						
31	33						- 30 to 35 feet BGS Drill Time : 2.75 minutes
32	33						
33	33						
34	33						
35	33						
36	33						- 35 to 40 feet BGS Drill Time : 3.08 minutes
37	38						
38	38						
	38						
39	38						
40	30						- 40 to 45 feet BGS Drill Time : 2.87 minutes
41	30						
42	36						
43	38						
44	38						
—— 45 46	38						- 45 to 49 feet BGS Drill Time : 2.53 minutes
46	38						
47	38						
48	38						Total Drill Time in Rocks : 20.69 minutes
49					9 feet BGS. entonite holeplug		
50	In-Borehole \					neral Notes	
,		Casing	Bot. of	Water	BGS = Below Grade Surface No groundwater observed.		
	Pate / Time	Tip (ft)	Hole (ft)	Lvl (ft)			





Location:

Percussion Drilling Log

PP - 08

Client: Concept Engineering Consultants Project:

Clinton Commons Clinton , NJ

Drilling Firm: Hayduk Enterprises Drill Crew: Garth Devlia

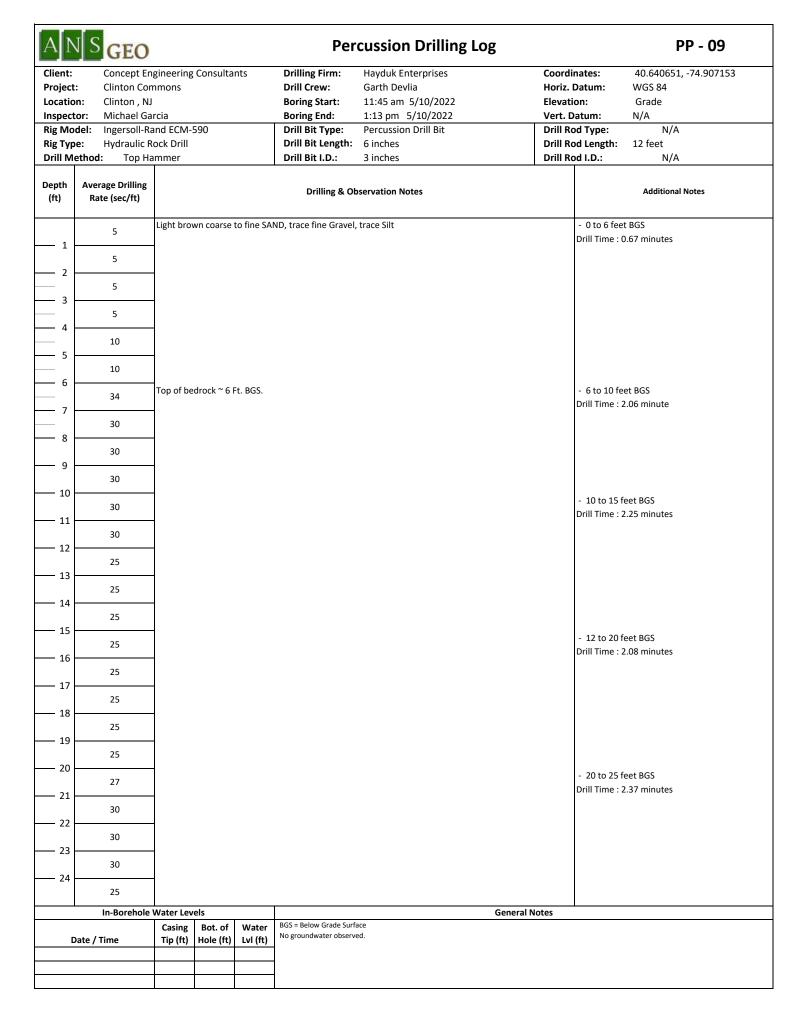
Boring Start: 11:00 am 5/04/2022

Coordinates: Horiz. Datum:

40.642493, -74.907857 WGS 84

Elevation: Grade

Inspect	or: Michael Ga	rcia			Boring End:	12:00 am 5/04/2022	Vert. D	Patum: N/A
Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & O	bservation Notes		Additional Notes
	20							- 25 to 30 feet BGS Drill Time : 1.7 minutes
26 27	20							Drill Tille 1.1.7 Tillinates
	20							
28	21							
29	21							
30	24							- 30 to 35 feet BGS Drill Time : 2.27 minutes
31	28							
33	28							
34	28							
	28							
—— 35 —— 36	28							- 35 to 40 feet BGS Drill Time : 2.73 minutes
	34							
37	34							
38 39	34							
	34							
40 41	35							- 40 to 45 feet BGS Drill Time : 2.77 minutes
—— 42 ·	35							
—— 43 ·	36							
44	30							
44	30							
—— 46	30							- 45 to 49 feet BGS Drill Time : 2.0 minutes
—— 47	30							
48	30							
—— 48 —— 49	30							Total Drill Time in Rocks : 15.1 minutes
50					19 feet BGS. entonite holeplug			
	In-Borehole	Water Lev	/els				General Notes	
D	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Sur No groundwater observ			
			1					





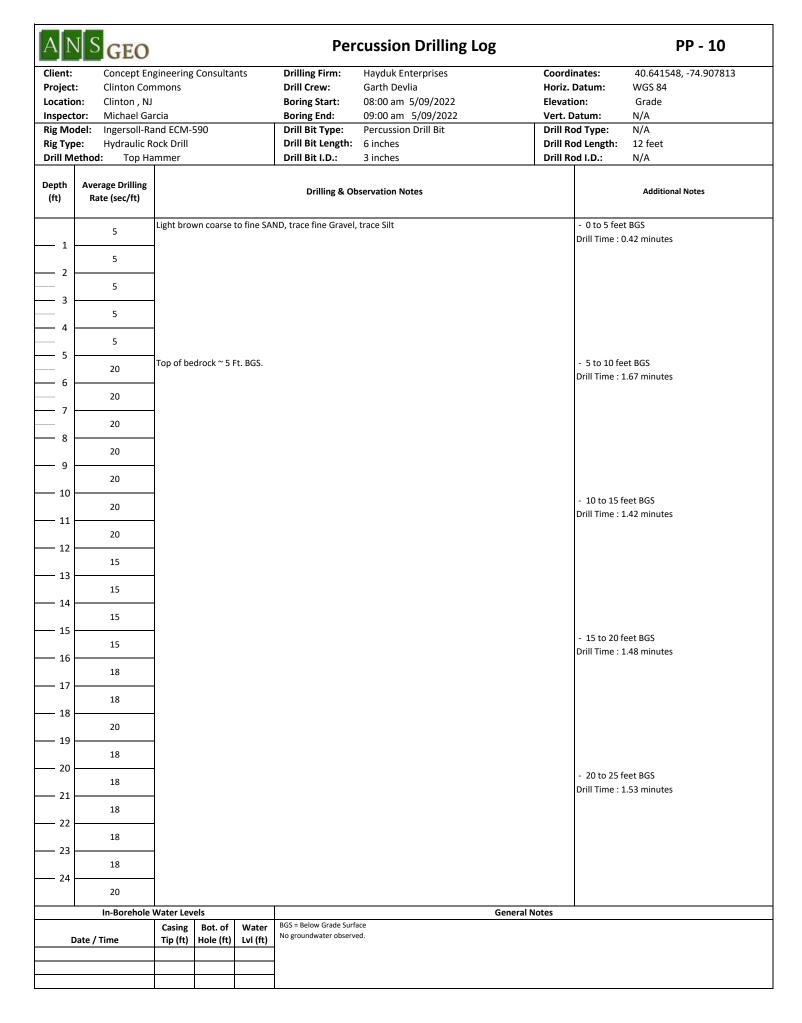
Percussion Drilling Log

PP - 09

ntinued)

Concept Engineering Consultants Drilling Firm: Hayduk Enterprises 40.640651, -74.907153 Client: Coordinates: **Drill Crew:** Garth Devlia WGS 84 Project: **Clinton Commons** Horiz. Datum: **Boring Start:** 11:45 am 5/10/2022 Location: Clinton , NJ Elevation: Grade 1:13 pm 5/10/2022 Inspector: Michael Garcia **Boring End:** Vert. Datum: N/A

Inspecto	or: Michael Ga	rcia			Boring End:	1:13 pm 5/10/2022	Vert. Dati	um: N/A	
Depth (ft)	Average Drilling Rate (sec/ft)				Drilling & C	Observation Notes		Additional Notes	
	25							25 to 30 feet BGS ill Time : 2.08 minutes	
26 _	25								
— 27 —	25								
— 28 -	25								
— 29 – — 30 –	25								
— 30 - — 31 -	30							30 to 35 feet BGS ill Time : 2.5 minutes	
- 32 -	30								
- 33 -	25								
- 34 -	30								
— 35 —	30								
— 36 –	30							35 to 40 feet BGS ill Time : 2.17 minutes	
- 37 -	25	1							
— 38	25								
— 39	25								
- 40	25							40 to 45 feet BGS	
— 41 -	30							ill Time : 2.08 minutes	
— 42	30								
— 43	25								
— 44 -	20								
— 45 —	20							45 to 49 feet BGS	
— 46 -	20						Dri	ill Time : 1.33 minutes	
— 47	20								
— 48 -	20							And Daill Times 1 D. J. 1997	
— 49 –					19 feet BGS.		Tot	tal Drill Time in Rocks : 19.59 mir	nutes
- 50	In-Borehole			iiigs and b	entonite holeplug		General Notes		
	iii-bureniile			144	BGS = Below Grade Su		General Mores		
Da	ate / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	No groundwater obser				





Percussion Drilling Log

PP - 10

ntinued)

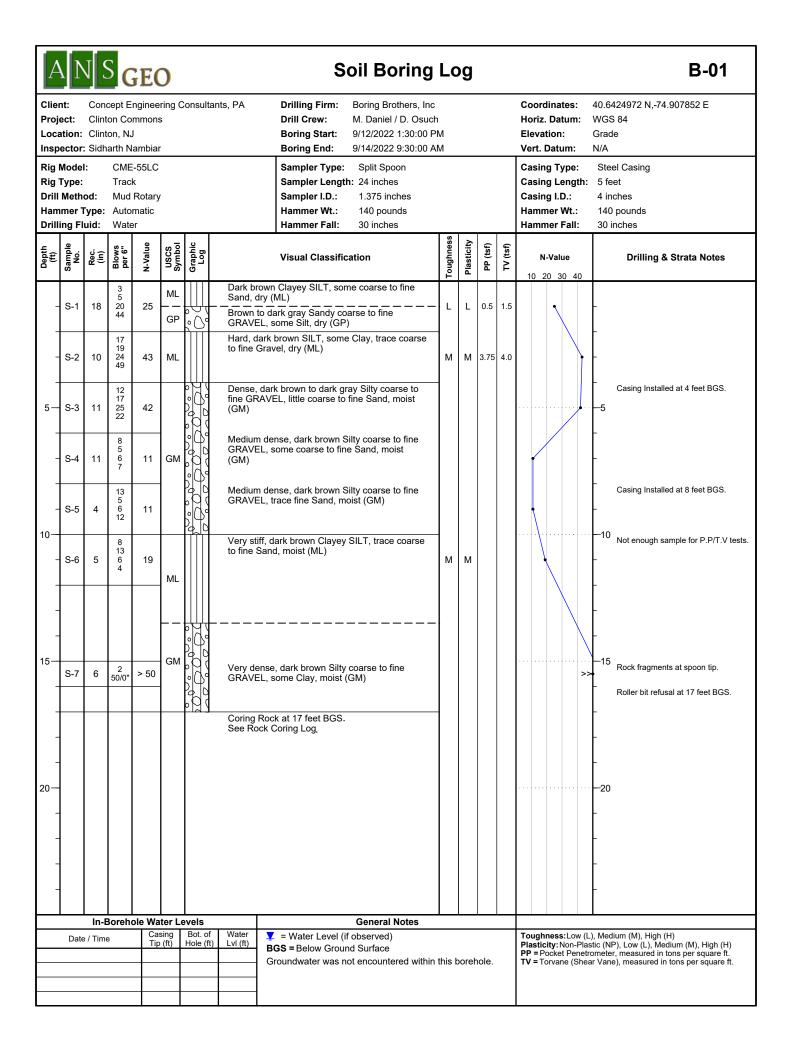
Drilling Firm: Hayduk Enterprises 40.641548, -74.907813 **Concept Engineering Consultants** Coordinates: Project: **Clinton Commons Drill Crew:** Garth Devlia WGS 84 Horiz. Datum: 08:00 am 5/09/2022 Location: Clinton , NJ **Boring Start:** Elevation: Grade Michael Garcia 09:00 am 5/09/2022 Inspector: **Boring End:** Vert. Datum: N/A

Depth Average Drilling (ft) Rate (sec/ft)					Additional Notes	
	20					- 25 to 30 feet BGS Drill Time : 1.8 minutes
_ 26 _	20					Dill fillie . 1.0 fillitutes
- 27 -	20					
- 28 -	24					
- 29 -	24					
– 30 –	22					- 30 to 35 feet BGS
- 31 -	20					Drill Time : 1.7 minutes
- 32 -	20					
- 33 -	20					
- 34	20					
- 35 -	20					- 35 to 40 feet BGS
- 36 -	25					Drill Time : 2.0 minutes
- 37 -	25					
- 38 -	25					
— 39 —	25					
— 40 —						- 40 to 45 feet BGS
- 41 -	20					Drill Time : 1.5 minutes
- 42 -	20					
- 43 -	15					
- 44 -	15					
– 45 –	20					- 45 to 49 feet BGS
– 46 –	20					Drill Time : 1.33 minutes
- 47	20					
– 48 –	20					
— 49 —	20	End of Po	rcussion F	Orilling at 1	9 feet BGS.	Total Drill Time in Rocks : 14.85 minutes
 50					entonite holeplug	
	In-Borehole \	Vater Lev				eral Notes
Da	ite / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	BGS = Below Grade Surface No groundwater observed.	



APPENDIX E

As-Completed Test Boring Logs





B-01

Concept Engineering Consultants, PA Drilling Firm: Boring Brothers, Inc Coordinates: 40.6424972 N,-74.907852 E Client: Project: **Clinton Commons Drill Crew:** M. Daniel / D. Osuch Horiz. Datum: WGS 84 9/12/2022 1:30:00 PM Location: Clinton, NJ **Boring Start:** Elevation: Grade 9/14/2022 9:30:00 AM Inspector: Sidharth Nambiar **Boring End:** Vert. Datum: N/A

Rig Model: CME-55LC Casing Type: Steel Casing Core Barrel Type: Core Bit Type: NQ - 01 Rig Type: Track Casing Length: 5 feet Core Barrel Length: 5 feet Core Bit Length: 3 inches Drill Mothods Coro Porrel I D . Coro Bit I D : Mud Rotary 1 875 inches

Drill N	lethod	: M	lud Ro	otary		Casing I.D.: 4 inches Core Barrel I.D.: 3 inches			Core	Bit	I.D.: 1.875 inches						
	"⊋				,	Бı		•				Dis	con	tinui	ties		
Depth (ft)	Avg Core Rate (min/ft)	Run No.	Recovery (in. / %)	RQD (in. / %)	Hardness	Weathering	Graphic Log	Visual Classification	on	Depth (ft.)	Туре	Dip Angle	Roughness	Weathering	Aperture	Infilling	Drilling & Strata Notes
	4.97							LIMESTONE, light gray fine grai weathered, very close to close d spacing.	ned, slightly iscontinuity								Casing Installed at 17 feet BGS.
	5.08							18.6' to 18.9' Highly Fractured Z	one.	18.2	J	50	P,R	FR	VT	N	Water loss at 18 feet BGS. Calcite veins throughout the cores.
-	5.17	R-1	58 97%	25 42%	R4	SL				19.8	J	20	P,Sm	DS	VT	N	Calcite veins throughout the cores. Light gray return. Vertical fracture at 6.1 feet BGS.
20-	4.83									20.9	J	20	P,Sm	FR	0	N	
-	4.92							20.9' to 21.8' Fractured Zone.		20.0	ľ	20	,0111				
	5.22							LIMESTONE, light gray fine grai weathered, close discontinuity s	ned, slightly pacing.	22.7	J	25	P,R	DS	VT	N	
	5.13																Water loss at 23 feet BGS.
25-	3.72	R-2	60 100%	48 80%				24.2 to 24.6 Fractured Zone.		24.2 24.7	J		P,R P,R		VT VT	N N	
	10.7									26	J	20	P,R	FR	VT	N	
_	9.97									20			. ,				
_								End of Boring at 27 feet BGS. Backfilled with soil and bentonite	e holeplug.								
30-																	
-	1																
-																	
-																	
-																	
35-																	
-																	
	In Republic Water Levels						C= 1		Ne4:								
I	In-Borehole Water Levels							Gen	eral	Note	S						

III-Borellole	. Water L	eveis	
Date / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)
			, in the second



Figure B-01.1 B-01; R-1 and R-2 (dry)

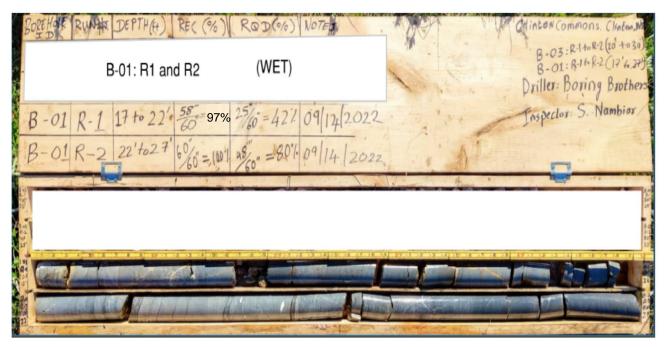
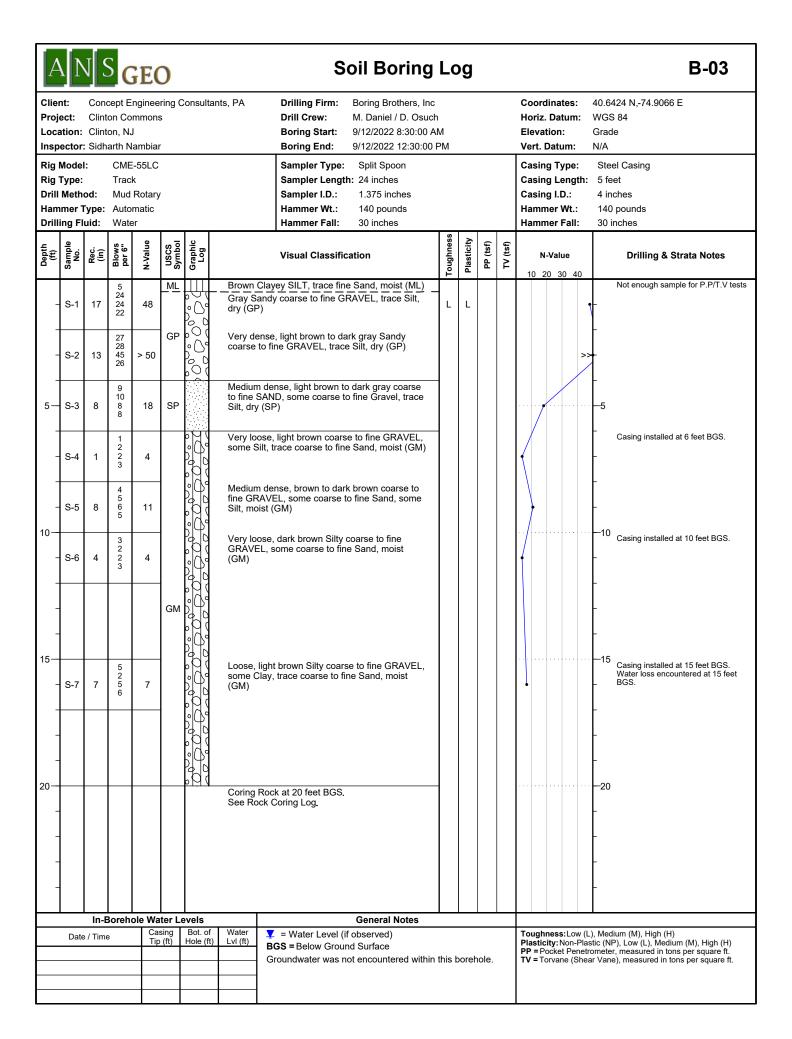


Figure B-01.2 B-01; R-1 and R-2 (wet)





B-03 Drilling Firm: Concept Engineering Consultants, PA Boring Brothers, Inc Coordinates: 40.6424 N.-74.9066 E Client: Horiz. Datum: **Drill Crew:** WGS 84 Project: M. Daniel / D. Osuch Clinton Commons 9/12/2022 8:30:00 AM Location: Clinton, NJ **Boring Start:** Elevation: Grade 9/12/2022 12:30:00 PM Inspector: Sidharth Nambiar **Boring End:** Vert. Datum: N/A Rig Model: CME-55LC Casing Type: Steel Casing Core Barrel Type: Core Bit Type: NQ - 01 Core Barrel Length: Core Bit Length: Rig Type: Track Casing Length: 5 feet 5 feet 3 inches **Drill Method:** 4 inches Mud Rotary Casing I.D.: Core Barrel I.D.: 3 inches Core Bit I.D.: 1.875 inches Discontinuities Weathering Recovery (in. / %) Graphic Log Dip Angle Depth **Visual Classification Drilling & Strata Notes** Weatheri LIMESTONE, light gray very fine grained, slightly weathered, very close to close discontinuity spacing. 3.25 3.22 20' to 25' Highly Fractured Zone. Water loss encountered at 22 feet 52 87% 0 0% R3 SL BGS. R-1 3.25 3.28 3.37 25 LIMESTONE, dark gray very fine grained, 4.75 slightly weathered, very close to close discontinuity spacing. 5.88 25' to 30' Highly Fractured Zone. SL R3 5.77 R-2 48% 0% 2.383 7.15 30 End of Boring at 30 feet BGS. Backfilled with soil and bentonite holeplug. 35 In-Borehole Water Levels **General Notes** Date / Time **BGS** = Below Ground Surface LvI (ft) Groundwater was not encountered within this borehole.

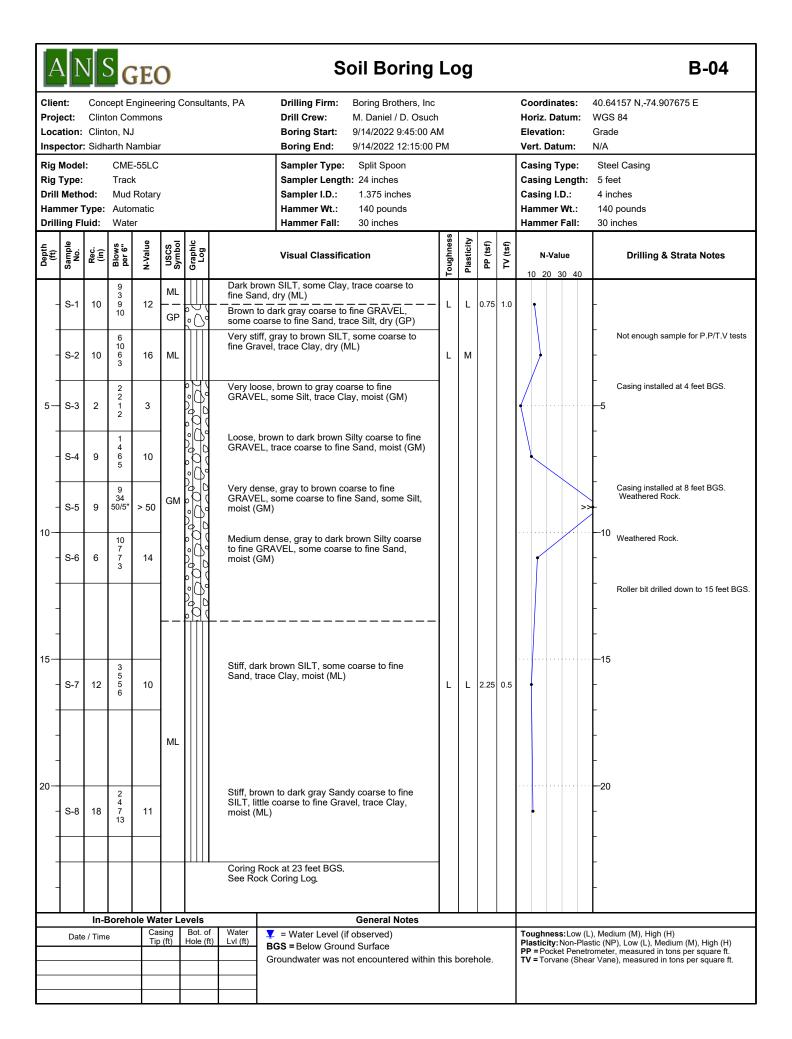
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Figure B-03.1 B-03; R-1 and R-2 (dry)



Figure B-03.2 B-03; R-1 and R-2 (wet)





B-04

Client: Concept Engineering Consultants, PA Drilling Firm: Boring Brothers, Inc Coordinates: 40.64157 N,-74.907675 E

Project: **Clinton Commons Drill Crew:** M. Daniel / D. Osuch Horiz. Datum: WGS 84 9/14/2022 9:45:00 AM Location: Clinton, NJ **Boring Start:** Elevation: Grade 9/14/2022 12:15:00 PM Inspector: Sidharth Nambiar Vert. Datum: **Boring End:** N/A

Rig Model: CME-55LC Casing Type: Steel Casing Core Barrel Type: Core Bit Type: NQ - 01 Rig Type: Track Casing Length: 5 feet Core Barrel Length: 5 feet Core Bit Length: 3 inches **Drill Method:** Mud Rotary Casing I.D.: Core Barrel I.D.: Core Bit I.D.:

Drill N	Drill Method: Mud Rotary Casin		Casing	I.D.: 4 inches Core Barrel I.D.:		3 inches					Core	Bit	I.D.: 1.875 inches				
	⊕€		>		v	β				Depth (ft.)		Dis	con	tinui	ties		
Depth (ft)	Avg Core Rate (min/ft)	Run No.	Recovery (in. / %)	RQD (in. / %)	Hardness	Weathering	Graphic Log		Visual Classification		Type	Dip Angle	Roughness	Weathering	Aperture	Infilling	Drilling & Strata Notes
	6.83							LIMESTONE, light gray fine grai weathered, very close to close d spacing.	ned, slightly iscontinuity								Casing installed at 23 feet BGS.
25-	5.95									24.9	J	40	P,R	FR	VT	N	
	4.37	R-1	46 77%	17 28%	R3	SL		23' to 24.6' Fractured Zone. 26.1' to 26.8' Fractured Zone.		25.3 25.8	J	45	P,R P,R	FR	VT T	N N	
-	5.85																
-	8.52							LIMESTONE, light gray fine grai	ned elightly								Casing installed at 28 feet BGS.
-	4.92							weathered, very close to close d spacing.	iscontnuity	28.8	J				T	N	Casing instance at 20 feet BOO.
30-	2.53									29.3 29.9 30.3	l l	40 20 10	P,R P,R P,R		T VT VT	N N N	
-	3.5	R-2	54 90%	33 55%	R4					30.8	J	20	P,R		VT	N	
-	3.37									32	J	50	P,R	DS	VT	N	
-	3.87							End of Boring at 33 feet BGS. Backfilled with soil and bentonite	holenlug								
-								backlined with soil and bentonite	s noispiug.								
35—																	
-																	
-																	
-																	
40																	
40-																	
	In-Borehole Water Levels							T				•					
	ı	n-Bor					Water			Gen	erai	Note	s				

III-Borelloi	e water L	eveis	
Date / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)

▼ = Water Level (if observed) BGS = Below Ground Surface Groundwater was not encountered within this borehole.



Figure B-04.1 B-04; R-1 and R-2 (dry)

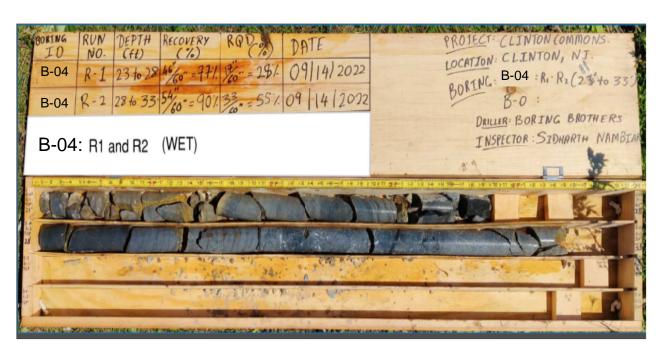
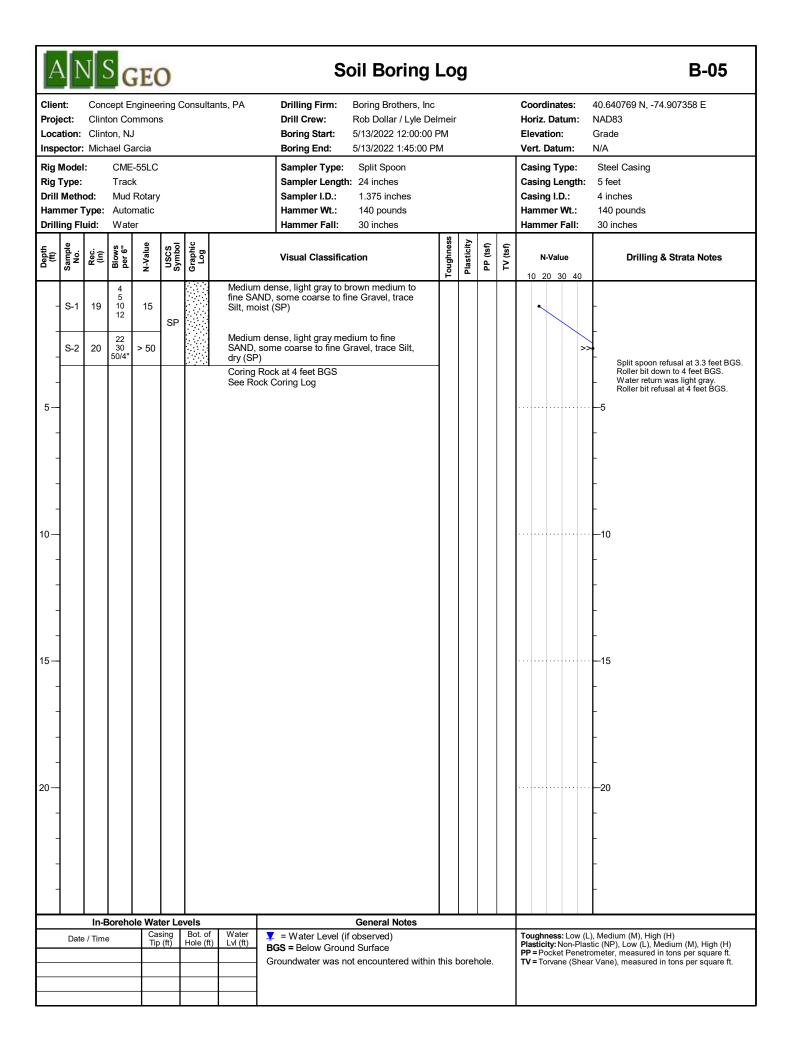
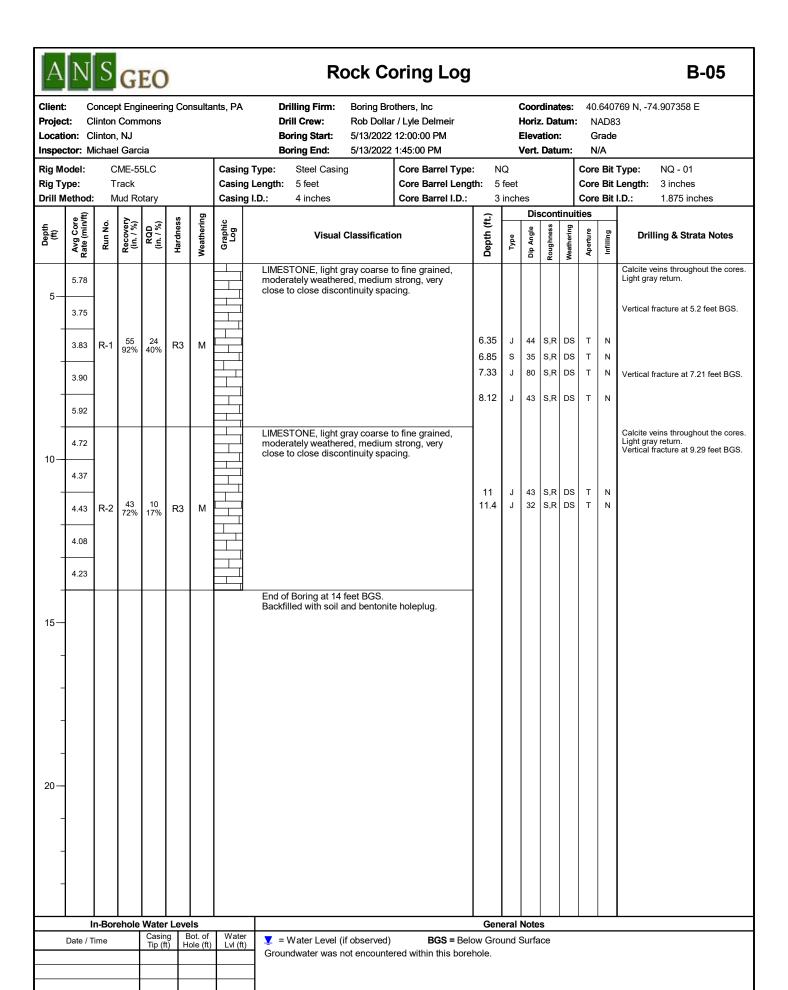


Figure B-04.2 B-04; R-1 and R-2 (wet)





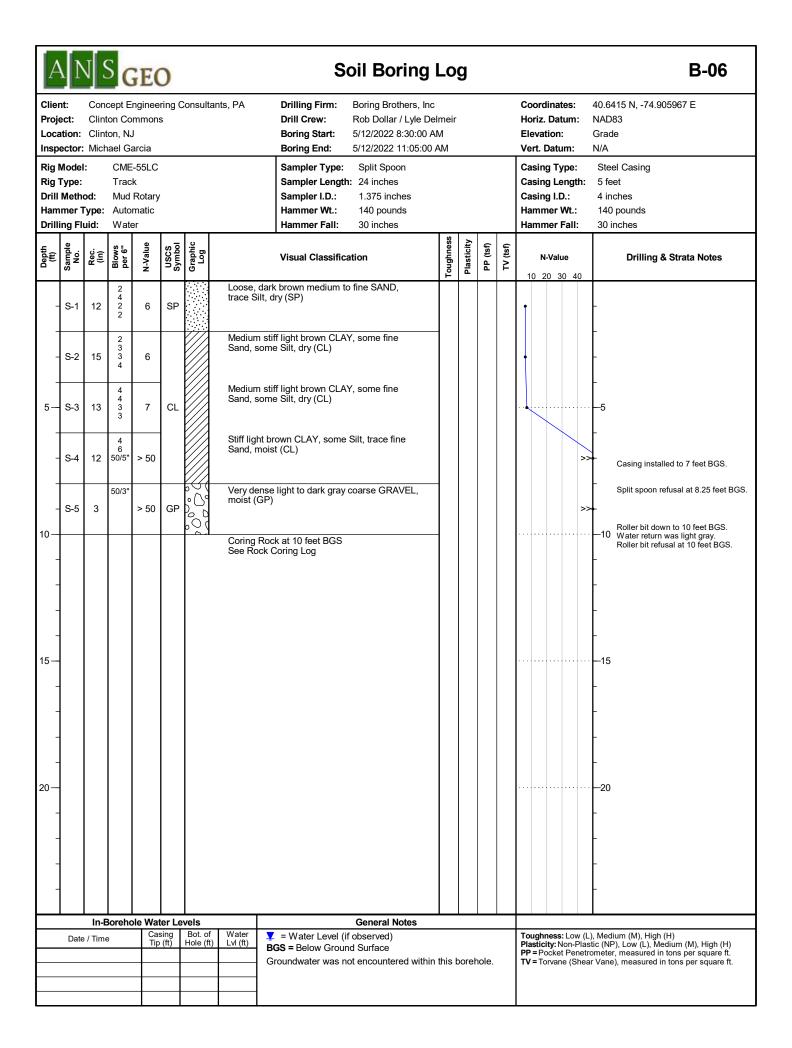


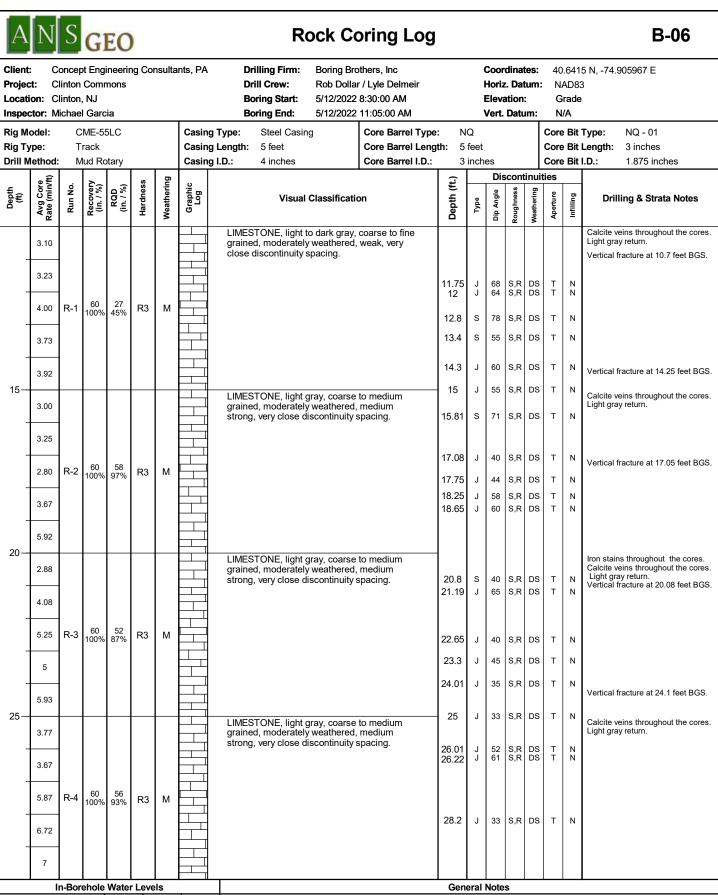


B-05; R-1 and R-2 (dry)



Figure B-05.2 B-05; R-1 and R-2 (wet)





▼ = Water Level (if observed)

BGS = Below Ground Surface

Groundwater was not encountered within this borehole.

Borehole camera was attempted, but unable to see due to color of water in the borehole.



Concept Engineering Consultants, PA **Drilling Firm:** Boring Brothers, Inc Coordinates: Client: 40.6415 N, -74.905967 E **Drill Crew:** Project: Rob Dollar / Lyle Delmeir Horiz. Datum: Clinton Commons NAD83 Location: Clinton, NJ 5/12/2022 8:30:00 AM **Boring Start:** Elevation: Grade 5/12/2022 11:05:00 AM Inspector: Michael Garcia **Boring End:** Vert. Datum: N/A Discontinuities Depth (ft.) Graphic Log Aperture Infilling Visual Classification **Drilling & Strata Notes** Туре ģ 30 60 DS LIMESTONE, light gray, coarse to medium Calcite veins throughout the cores. grained, moderately weathered, medium 30.5 40 P,R DS VT Ν Light gray return. strong, very close discontinuity spacing. 3.42 34 57% R3 R-5 3.52 31.3' to 34.4' Fractured Rock. 3.32 60 S,R DS 33.75 34.1 50 S,R DS Т Ν 30 S,R DS Ν 34.45 Т 3.5 35 35 40 S,R DS Т Ν LIMESTONE, light gray, coarse to medium grained, moderately weathered, medium strong, very close discontinuity spacing. Calcite veins throughout the cores. 35.5 s 50 S,R DS Т Ν 3.15 2.63 49 3.50 R-6 R3 Μ 37.75 S,R DS Т Ν 50 38.4 30 S,R DS Ν 4.17 4.07 End of Boring at 40 feet BGS. Backfilled with soil and bentonite holeplug. 45-50 In-Borehole Water Levels **General Notes** Water Lvl (ft) Date / Time ▼ = Water Level (if observed) **BGS** = Below Ground Surface Groundwater was not encountered within this borehole.





Figure B-06.1 B-06; R-1, R-2, R-3 and R-4 (dry)



Figure B-06.2 B-06; R-1, R-2, R-3 and R-4 (wet)

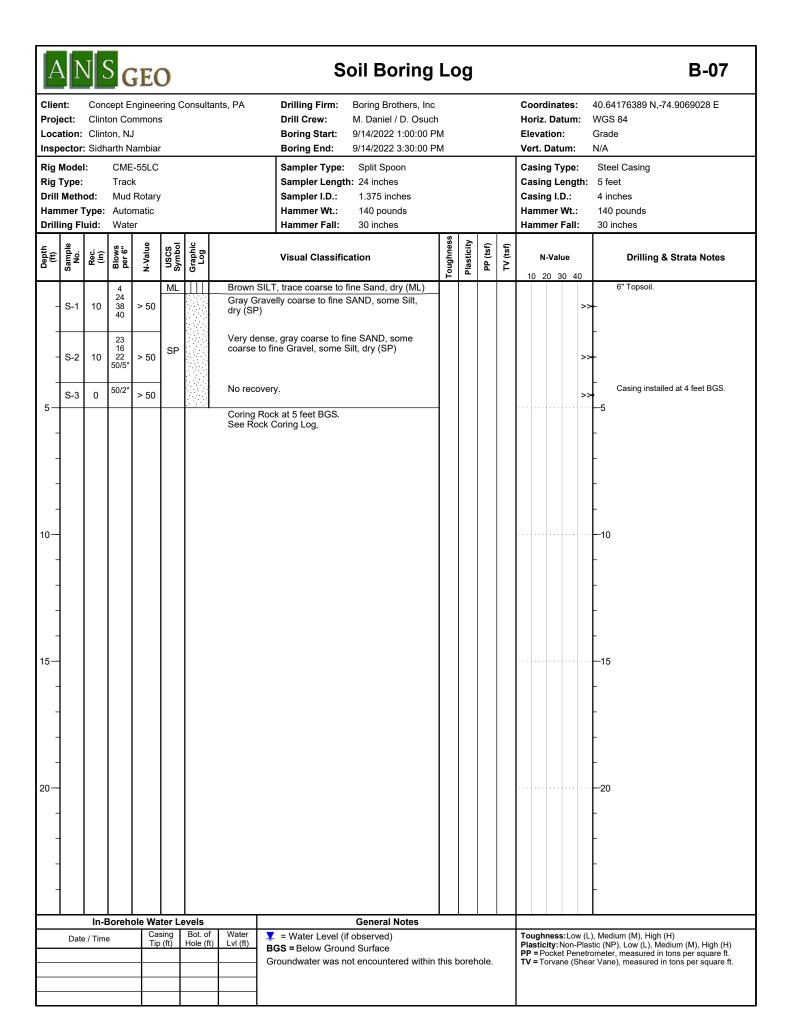


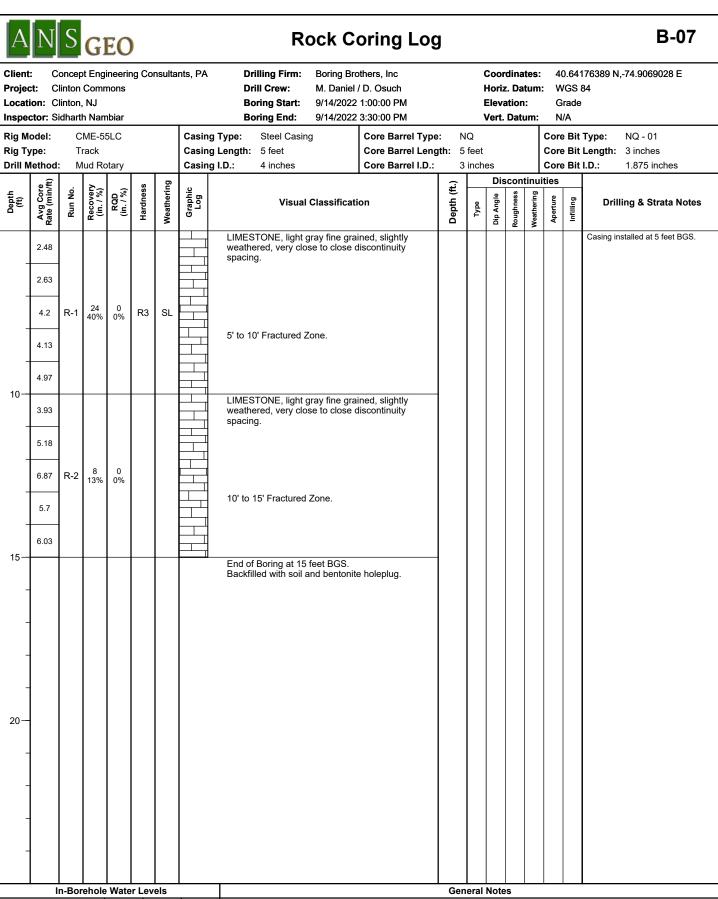


Figure B-06.3 B-06; R-5 and R-6 (dry)



Figure B-06.4 B-06; R-5 and R-6 (wet)





In-Borenoi	e water L	.eveis		L
Date / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)	
				l
				l

Groundwater was not encountered within this borehole.

BGS = Below Ground Surface

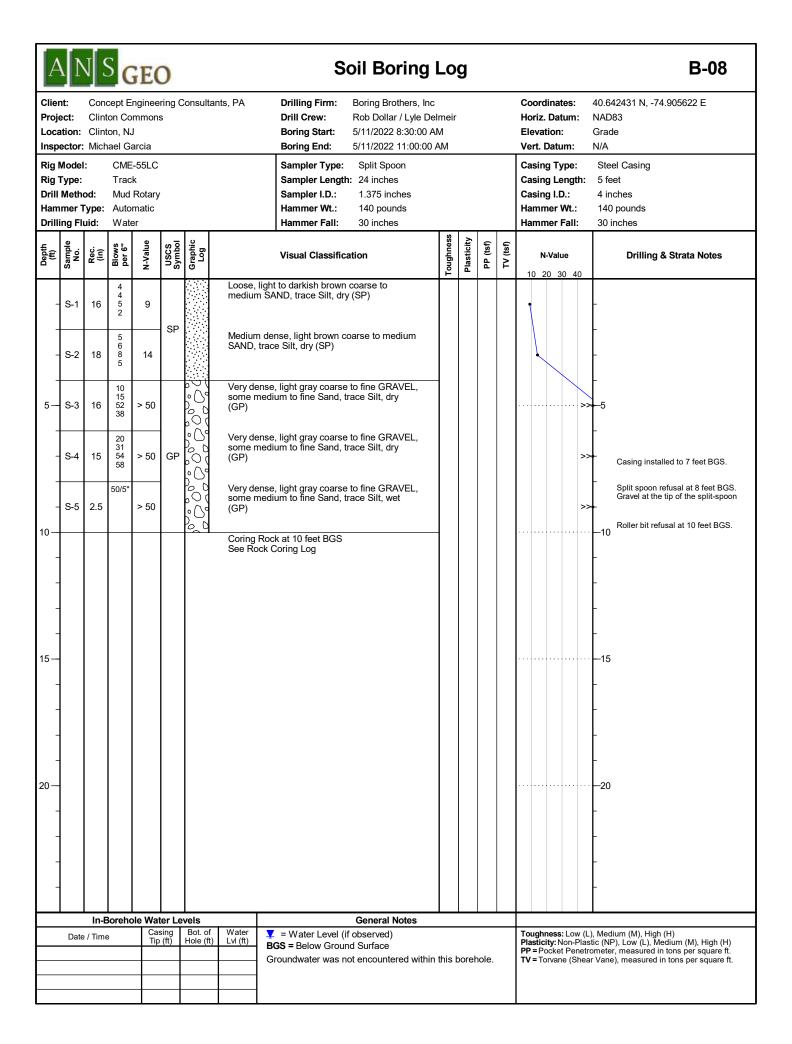




Figure B-07.1 B-07; R-1 and R-2 (dry)



Figure B-07.2 B-07; R-1 and R-2 (wet)



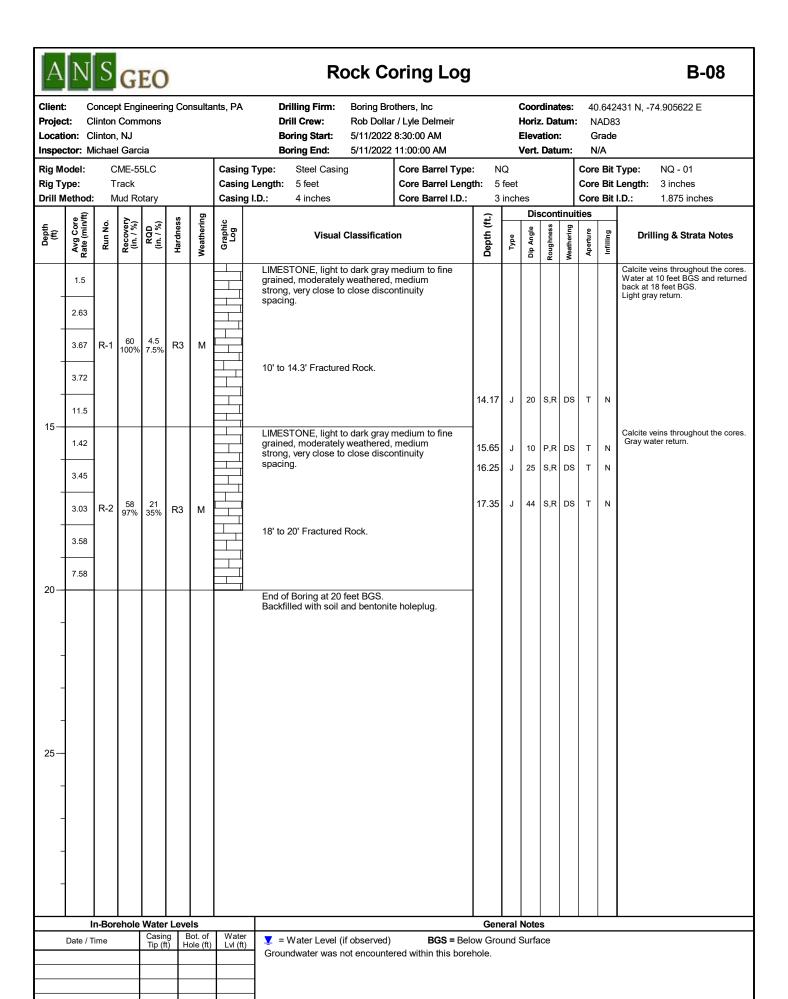


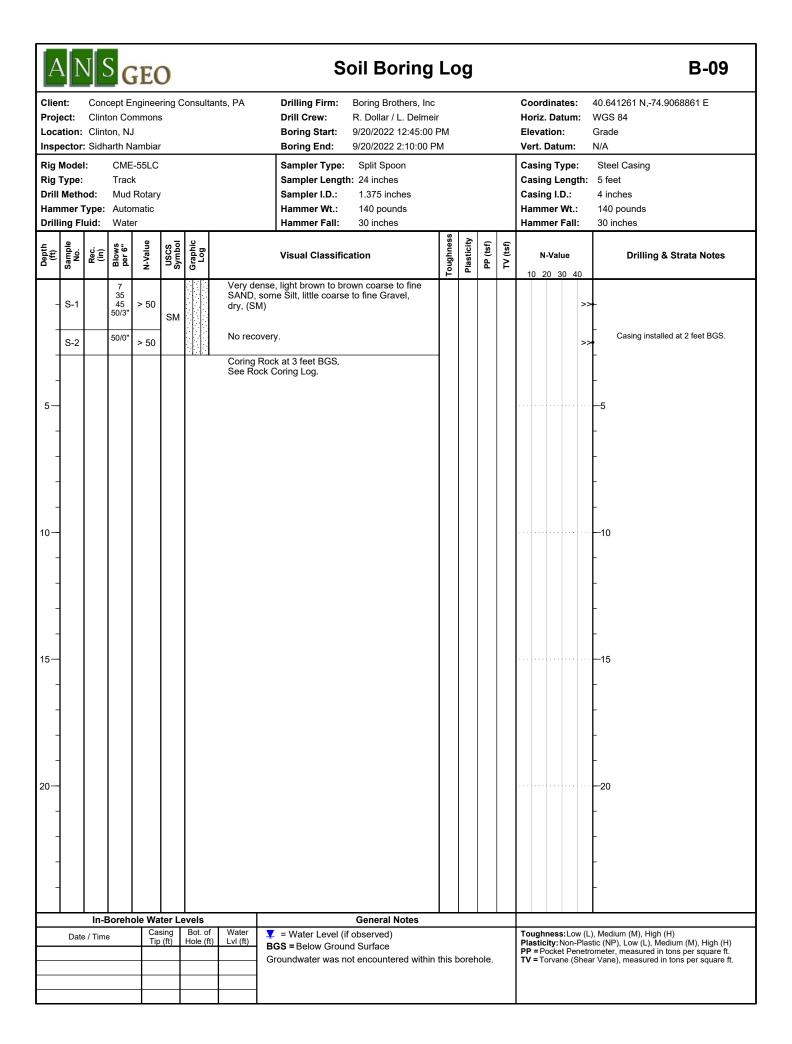




Figure B-08.1 B-08; R-1 and R-2 (dry)



Figure B-08.2 B-08; R-1 and R-2 (wet)





B-09

Client: Concept Engineering Consultants, PA Drilling Firm: Boring Brothers, Inc Coordinates: 40.641261 N,-74.9068861 E

Project:Clinton CommonsDrill Crew:R. Dollar / L. DelmeirHoriz. Datum:WGS 84Location:Clinton, NJBoring Start:9/20/2022 12:45:00 PMElevation:GradeInspector:Sidharth NambiarBoring End:9/20/2022 2:10:00 PMVert. Datum:N/A

Rig Model: CME-55LC Casing Type: Steel Casing Core Barrel Type: Core Bit Type: NQ - 01 Rig Type: Track Casing Length: 5 feet Core Barrel Length: 5 feet Core Bit Length: 3 inches **Drill Method:** Mud Rotary Casing I.D.: Core Barrel I.D.: Core Bit I.D.:

Drill N	Drill Method: Mud Rotary			Casing I	I.D.: 4 inches	Core Barrel I.D.:	3 inches				Core	Bit	I.D.: 1.875 inches				
	£)		Γ,			ıg						Dis	scon	tinui	ties		
Depth (ft)	Avg Core Rate (min/ft)	Run No.	Recovery (in. / %)	RQD (in. / %)	Hardness	Weathering	Graphic Log	Visual Classification		Depth (ft.)	Туре	Dip Angle	Roughness	Weathering	Aperture	Infilling	Drilling & Strata Notes
	3.1							LIMESTONE, light gray fine grain weathered, very close discontinu	3.7	J	40	P,R	FR	VT	N		
5	1.68									4.5	J	55	P,R	FR	VT	N	
_	1.383	R-1	35 58%	12 20%	R3	SL											
_	2.25							5.1' to 5.9' Fractured Zone.									
_	4.97							LIMESTONE, light gray fine grain	and alightly								
-	5.07							weathered, very close to close di spacing.	iscontinuity	8.6	J	30	P,R	DS	VT	N	
10-	2.17				Da	CI.		8.6' to 13' Fractured Zone.									
-	3.07	R-2	R-2 48 6 R3 SL			6 R3 SL											
-	3.75																
-	4.35							End of Boring at 13 feet BGS.									
_								Backfilled with soil and bentonite	noiepiug.								
15-																	
-																	
-																	
-																	
-																	
20—																	
-																	
-																	
	In-Borehole Water Levels								•	Gen	eral	Note	s				

Date / Time Casing Hole (ft) Water Hole (ft) Lvl (ft)

▼ = Water Level (if observed) BGS = Below Ground Surface Groundwater was not encountered within this borehole.



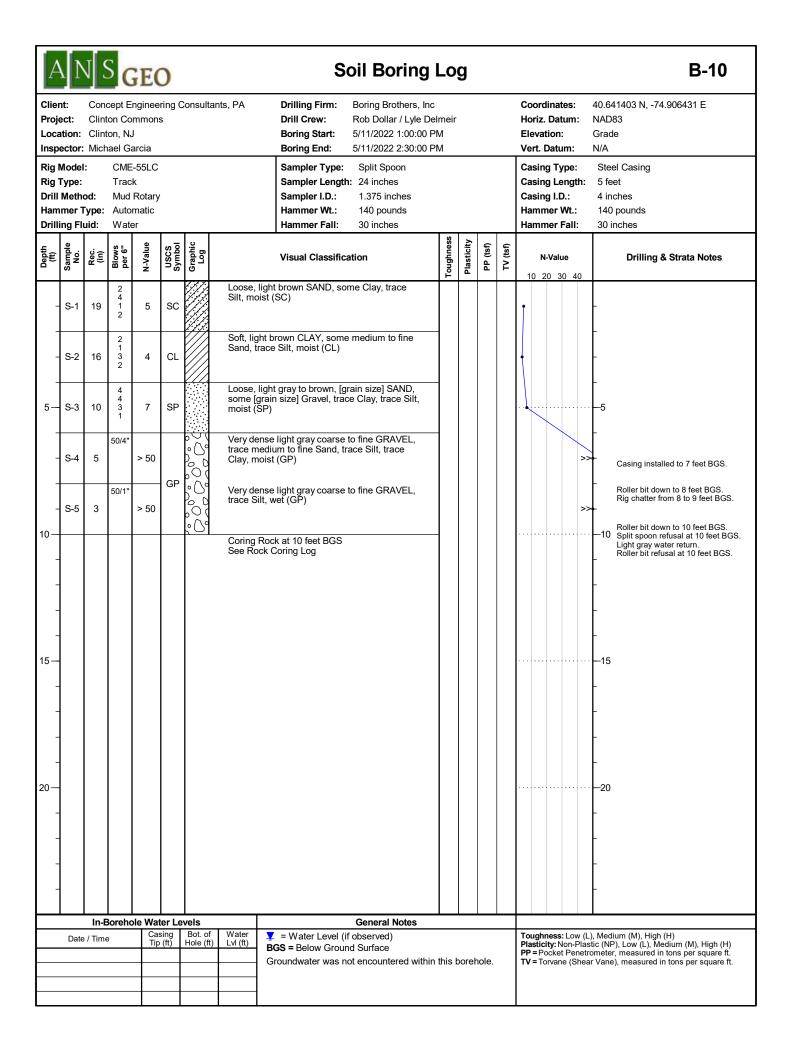
Core Photo Log

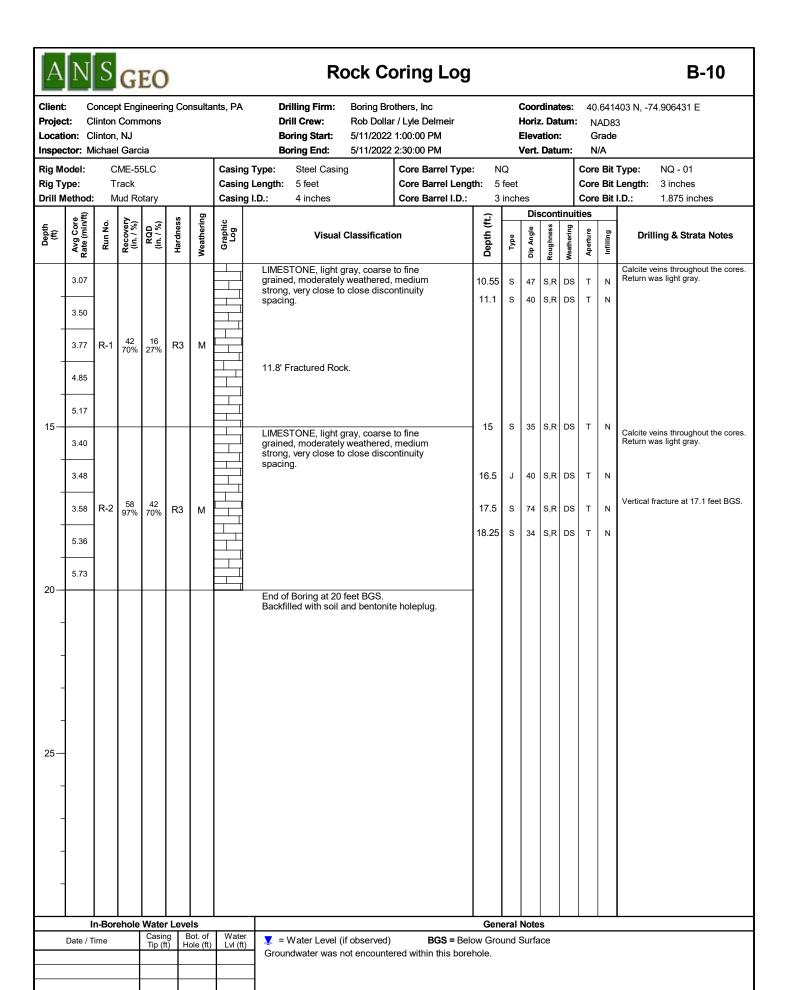


Figure B-09.1 B-09; R-1 and R-2 (dry)



Figure B-09.2 B-09; R-1 and R-2 (wet)





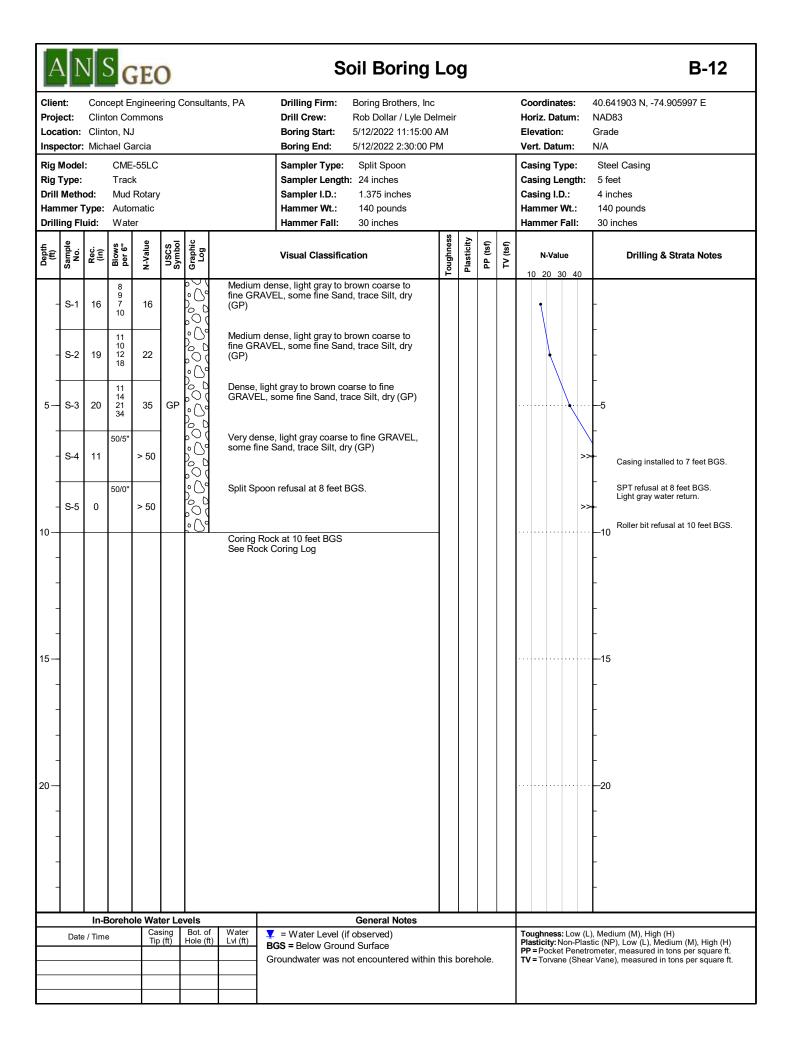


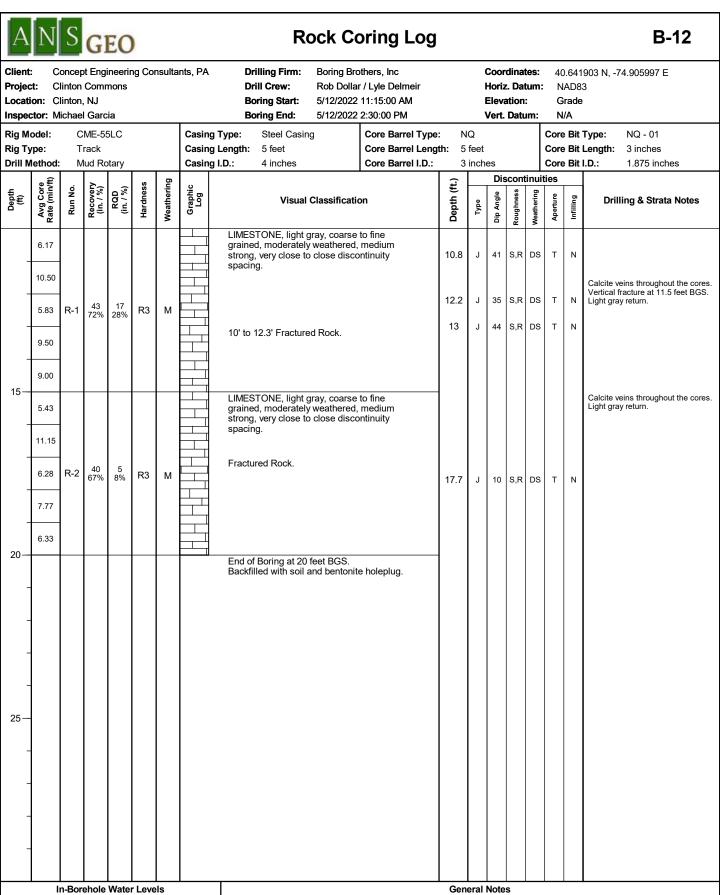


B-10; R-1 and R-2 (dry)



Figure B-10.2 B-10; R-1 and R-2 (wet)





III-Borchole Water Levels										
Date / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)							

▼ = Water Level (if observed)

Groundwater was not encountered w

BGS = Below Ground Surface

Groundwater was not encountered within this borehole.

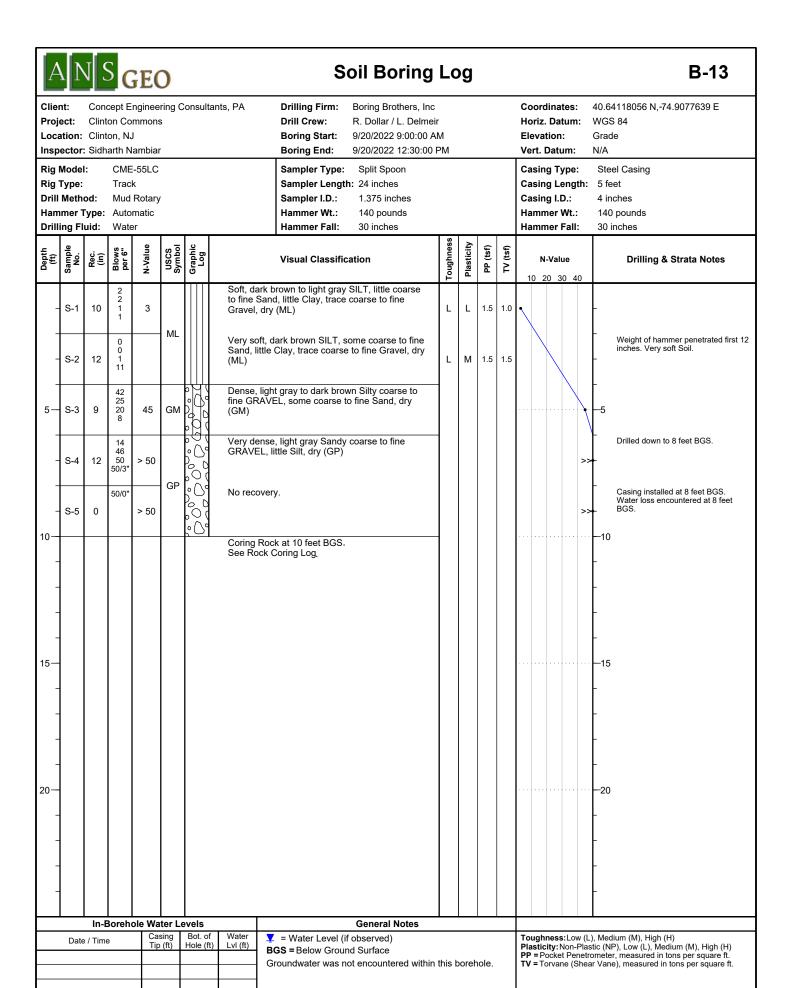




B-12; R-1 and R-2 (dry)



Figure B-12.2 B-12; R-1 and R-2 (wet)





Rock Coring Log

B-13

Client: Concept Engineering Consultants, PA Drilling Firm: Boring Brothers, Inc Coordinates: 40.64118056 N,-74.9077639 E

Project: **Clinton Commons Drill Crew:** R. Dollar / L. Delmeir Horiz. Datum: WGS 84 9/20/2022 9:00:00 AM Location: Clinton, NJ **Boring Start:** Elevation: Grade Inspector: Sidharth Nambiar **Boring End:** 9/20/2022 12:30:00 PM Vert. Datum: N/A

Rig Model: CME-55LC Casing Type: Steel Casing Core Barrel Type: Core Bit Type: NQ - 01 Rig Type: Track Casing Length: 5 feet Core Barrel Length: 5 feet Core Bit Length: 3 inches **Drill Method:** Mud Rotary Casing I.D.: Core Barrel I.D.: 3 inches Core Bit I.D.: 1.875 inches 4 inches

		Casing	sing I.D.: 4 inches Core Barrel I.D.:		3 inches				Core Bit		I.D.: 1.875 inches						
h h lo. lo. lo. %)		<u>e</u>		Disconting													
Depth (ft)	Avg Core Rate (min/ft)	Run No.	Recovery (in. / %)	RQD (in. / %)	Hardness	Weathering	Graphic Log	Visual Classificati	on	Depth (ft.)	Type	Dip Angle	Roughness	Weathering	Aperture	Infilling	Drilling & Strata Notes
_	1.93	_						LIMESTONE, light gray fine grai weathered, very close discontinu	ned, slightly uity spacing.	10.8	J	50	P,R	FR	VT	N	Water loss encountered.
_	1.6																
_	4.95	R-1	28 47%	5 8%	R4	SL		10' to 15' Fractured Zone.									
-	7.12							10 to 15 Hactured 25he.									
15-	2.37							LIMESTONE, light gray fine grai weathered, very close discontinu	ned, moderately								
-	2.07							weathered, very close discontinu	uity spacing.	16.2	J	30	P,R	FR	VT	N	
-	1.95	R-2	57 95%	43 72%		М				17.6	J	40	P,R	DS	VT	N	
-	1.87		95%	1270						17.0		40	,,,,	БО	٧١	1	
-	1.38									19 19.8	J		P,R P,R			N N	
20-							<u> </u>	End of Boring at 20 feet BGS. Backfilled with soil and bentonite	holeplug.	10.0		00	,,,,	БО	•	.,	
_																	
_																	
25-																	
-																	
_																	
-																	
-																	
In-Borehole Water Levels										Gen	eral	Note	s				

iii-Dolellole water Levels										
Date / Time	Casing Tip (ft)	Bot. of Hole (ft)	Water Lvl (ft)							

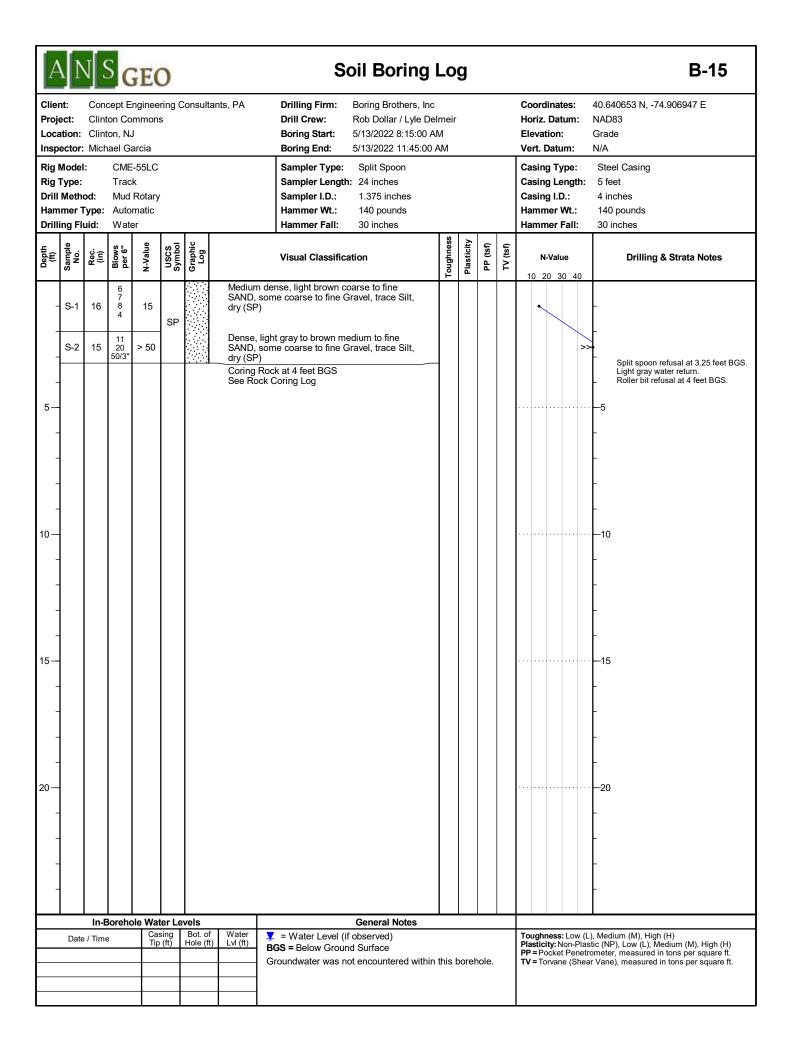
▼ = Water Level (if observed) BGS = Below Ground Surface Groundwater was not encountered within this borehole.



Figure B-13.1 B-13; R-1 and R-2 (dry)



Figure B-13.2 B-13; R-1 and R-2 (wet)



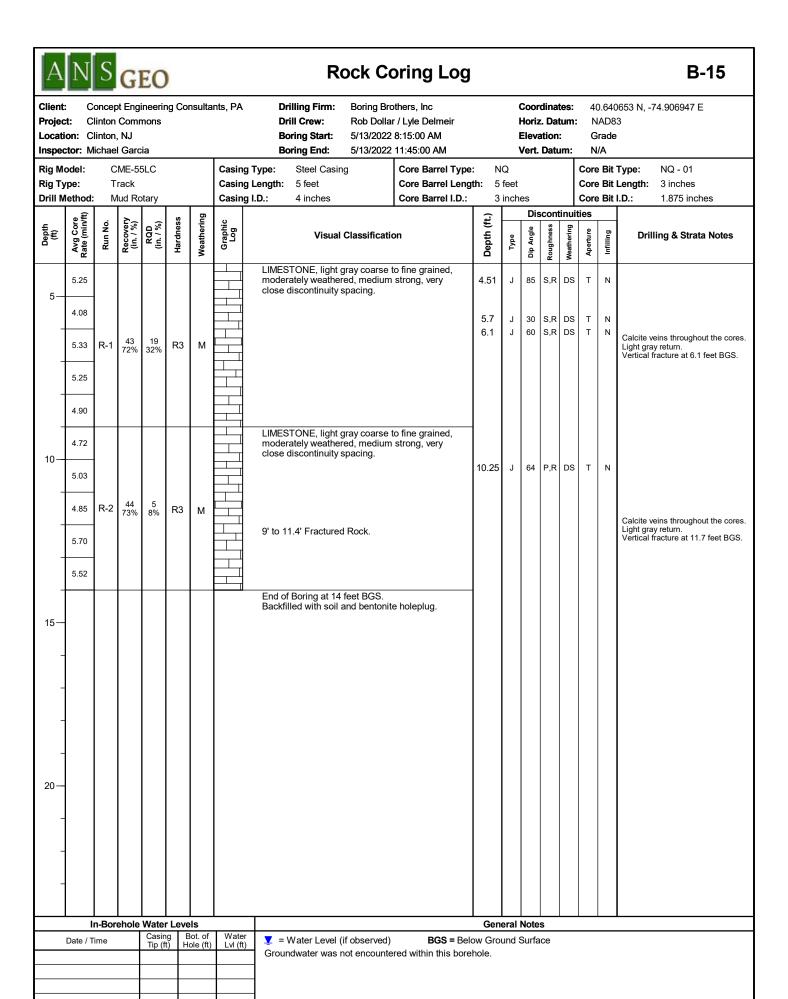






Figure B-15.1 B-15; R-1 and R-2 (dry)



Figure B-15.2 B-15; R-1 and R-2 (wet)



